#### earthquake damage report



project	Pacific Tower, 166 Gloucester Street	project no	5631
date	19 <sup>th</sup> May 2011	from	Sean Gardiner
client	Pacific Tower Body Corporate		

#### 1 Scope of this Report

This report covers our assessment of the structural condition of Pacific Tower located at 166 Gloucester Street, Christchurch on the 31<sup>st</sup> March 2011 based on a visual inspection inside and out and isolated removal of internal linings to expose the steel superstructure.

Our earlier initial inspection dated 25<sup>th</sup> February 2011 mainly addresses initial safety matters relating to the building. This subsequent inspection and report describes the damage observed in more detail, and comments on remedial work options for repair of the building as well as further suggested investigations.

This report does not cover a detailed structural strength assessment; which we consider is not warranted given its recent construction and minimal structural damage.

#### 2 Scope of Investigation

On the 31st March 2011, we visually inspected the building including:

- The exterior from ground level
- The interior including:
  - Stairwells
  - Floor levels 1, 2 (including carstackers), 6, 7, 11, 16 and 22
  - Steel frames exposed at the above levels

This report is based on our assessment of the building at the time stated. Photos that are attached are indicative of the damage. Any subsequent loading by aftershocks, or high winds, may initiate further damage.

#### 3 Building Description

Pacific Tower consists of a twenty-two storey, steel framed building with precast concrete cladding panels for most of its height. It has a reinforced masonry block lift machine room centrally on the roof.

The lower portion of the building (to the underside of level 6) is clad on the north and west sides with stone, with the south end of the building constructed from reinforced masonry. Part of the western elevation cantilevers over a Right Of Way access to the basement of Cathedral Junction to the south.

The floors and roof are typically constructed with a 150mm thick reinforced concrete topping on steel Comflor 80 flooring supported on composite steel beams.

The building is supported laterally with a combination of steel Eccentrically Braced Frames (EBF), in both 'K' and 'D' configurations, and Moment Resisting Frames (MRF). There are transfer diaphragms at levels 2, 6 and 11 to maintain EBF continuity.



Reinforced concrete pile caps and foundation beams are supported on a combination of bored (caisson) concrete piles, and steel screw piles (used primarily for tension loads).

#### 4 Damage Description

The building has suffered minor structural and non-structural damage as noted below:

- Onset of yielding and some minor permanent deformation of the active links of EBF's, refer following section on detailed frame investigations. No damage was observed to steel MRFs, columns, braces or welded or bolted connections.
- The lift technicians have measured the vertical alignment of the lift shafts and have observed the following worst case permanent displacements:
  - o 66mm to the south relative to the base at around L11/L12, returning to around 38mm offset at L18 and above.
  - 50mm to the east relative to the base at around L11/L12, returning to around 25mm offset at L19 and above.

These offsets are not of structural concern as they are well within the displacements allowed for during the design of the building.

- Isolated damage to a precast panel connection was noted and while not causing immediate concern, warrants further inspections (by way of exterior survey of the panels, and exposure of a sample of interior connections). Two fly-brace connections at panel fixing locations are missing their anchors to the floor at level 2 (east wall at top of ramp).
- Movement of podium level stone cladding to engage veneer ties, though there does not appear to be any connection failures. These should also be inspected as part of exterior survey.
- The concrete panels will have likely suffered further minor cracking and isolated spalling to that noted after the 4<sup>th</sup> September 2010 (refer previous exterior survey and subsequent repair specification sent on 7<sup>th</sup> October 2010). The extent of the cracking/spalling should also be verified during the external survey. Panels that have "bound" should be sawcut free, so there is no connection between adjacent panels, and the joints resealed. Otherwise, the sliding panel connections appear to have performed well as expected.
- The soffit linings to some of the balconies on the north face have fallen off, and there have been locations of further damage to the tiled junction between the balcony deck and the precast cladding panels. These should be repaired as previously specified with gaps to allow vertical seismic movements to occur.
- Fire rated Gib linings (non-structural) are severely cracked throughout both stairwells for the full height of the building. Subject to a report from the fire engineer these stairwells are not likely to comply with the Building Code requirements for Egress from Fire due to the Gib damage. The sliding stair details for the building appear to have performed well structurally. There is minor damage to the stair landings including isolated spalling of floor levelling compound where the central timber framed wall has impacted.
- The cross-bracing in many of the car stackers has unhooked at midway along its length. These will require repair/reinstatement prior to carstacker operation. (We suggest "D" shaped shackles be used to prevent this happening again). Inspection of



the carstacker restraints at the top of the carstackers by Kiwi Sparky (electrical contractor) have identified two fractured SHS restraints and some cleat connections that have pulled out, which will require repair.

- The seismic flashings between the south reinforced masonry block area and the main tower have been crushed full height and will require replacement. It is likely there is isolated roof damage as well. Flashings have also been dislodged at the junction with the Sampan Noodle House to the east, and may have suffered damage at the corners of the building.
- There is minor damage at the lift landing areas and we have been advised there has been damage to the lift shafts and guide rails rendering the lifts partially inoperable. The minor permanent displacement of the building will require the guide rail brackets to be re-aligned and the lift door locations to be adjusted at most levels. We understand a lift technician's report will confirm the full extent of the damage.
- Most rooms have damage similar in nature to the 4<sup>th</sup> September and Boxing Day quakes including cracked wall and ceiling linings, tiles, glass doors and wardrobes.
   There is also jamming of several hotel room doors preventing closure or access.
- We understand there is no emergency lighting operating in the building.

In addition, the required investigation works to verify the structural condition of the braced frames following the earthquake have lead to damaged wall and ceiling Gib linings and compromised the fire-rating in some locations.

We understand a report on damage to the fire systems is also being completed by the fire engineer.

#### 5 Detailed frame investigations

The linings were removed (where necessary) and the steel frames (EBF active links and MRF Potential Plastic Hinge Region locations) visually inspected for yielding and any significant permanent offset or fracture at the flowing locations (based on original construction grids):

Level 1: Grid E1(MRF), F2(MRF) Level 2: Grid E½1, F5, C4(west)

Level 6: Grid C2, C4(west), D4(east), C½6
Level 7: Grid C2, C4(west), D4(east), C½6, F5
Level 11: Grid C2, C4(west), D4(west & east), C½6
Level 16: Grid C4(west), D4(west & east), C½6
Level 22: Grid C½4, C½6, D4(east), E(MRF)

These locations were selected due being at transfer diaphragms levels as well as being at the levels of greatest observed non-structural damage (jamming doors, etc), suggesting greatest movement. Generally all of the readily accessible links at each selected level were inspected. It is anticipated the viewed active links would be relined with (fire rated) access panels to allow easier future inspection.

There was evidence of the onset of yielding, with diagonal Lueder's lines and paint flaking being evident in some active link regions. Only one link had any significant permanent displacement (at L2, grid E½1) and which had paint flaking at the ends of the braces as well as the active link region. This beam, adjacent to a carstacker, has no slab on top and was restrained laterally by a 327 HCC beam on its side (similar to the ones directly above).



Interim advice on criteria for actions on EBF inelastically responding active links from Charles Clifton and others from the University of Auckland suggests "where the maximum applied strain is in the order of +/- 2 TO 5% for a small number of load cycles, for example 3, then there is no need for metallurgical treatment of the steel". An upper limit of +/- 5% peak strain has been set to ensure that the toughness requirements of NZS3404 are still able to be met. With strains in these regions "...all the indications point to the steel retaining sufficient ability to absorb damage to withstand, at least, another ultimate limit state earthquake."

Inter-storey drifts, based on visible "scuff" marks, localised crushing if Gib, etc, appear to be in the order of 15mm. The applied strain, based on the damaged observed, is estimated to be within the range 2-5%. This estimate should also be compared with that calculated by the method noted in section 8.

No damage or sign of any permanent displacement was observed to steel MRFs, columns, braces or welded or bolted connections.

#### 6 Structural Safety Evaluation of Building

We did not observe any apparent structural safety hazards.

#### 7 Temporary Securing of the Building

As the building is new, has suffered no significant structural damage and there are no observed falling hazards, we consider that no temporary securing work is required.

#### 8 Long Term Repair

The following further investigation work should be undertaken:

- Exterior survey of precast panels and stone cladding. Please note extents and locations of any concrete cracking and spalling, locations where panels have "bound", as well as any panels that are now misaligned which may indicate damage/failure of a panel connection. The panel fixings at these locations should be exposed on the inside face for inspection.
- Ongoing correspondence with Charles Clifton (University of Auckland) has suggested a further assessment and estimate of the inelastic demand on the active links may be warranted. This would require a measurement or estimate of the interstorey displacements at the frame locations followed by a desktop study based on the frame geometries. We can conduct this assessment early in the repair phase.

The following repair work should be undertaken based on our observations to date:

- Repair damaged panels/balconies. Drill and epoxy inject cracks 0.2mm wide or greater with Sika Injectokit system. Finish with a faring coat. Cracks smaller than 0.2mm may be painted with a flexible brushable crack filler. Spalled sections of concrete should be repaired with the Sika Monotop system (primer, structural mortar and faring coat). Install in accordance with manufacturers literature. Panels that have "bound" should be sawcut free, so there is no connection between adjacent panels, and any damaged joints resealed.
- Repair stair landings with Sika Monotop system as above and floor levelling compound. Please provide 20mm seismic gap at ends of the lightweight/Gib lined stairwell central wall at landing locations and fill with fire rated sealant.
- Reinstate carstacker cross braces and top lateral restraints. Re-weld fractured restraints with FPBW, examine other restraints for signs of damage and



repair/reinforce welds as required. Replace and re-fix anchors which have pulled out of the blockwork masonry.

- Install missing anchors between braces and floor at L2 (top of ramp) on east wall.
- Repair non-structural damage to roofing, flashings, tiles, door frames, ceilings, linings, etc in accordance with manufacturer's recommendations.
- Repair lifts and fire systems in accordance with lift technician's report and fire engineer's report respectively.
- Repaint flaked intumescent paint to exposed steel beams at L2 (prepare steel in accordance with manufacturer's recommendations).

#### 9 Limitations

Findings presented as part of this report are for the sole use of the client. The findings are not intended for use by other parties, and may not contain sufficient information for the purposes of other parties or other uses. Our professional services are performed using a degree of care and skill normally exercised, under similar circumstances, by reputable consultants practicing in this field at this time. No other warranty, expressed or implied, is made as to the professional advice presented in this report.

Report by:

Sean Gardiner

B.E.(Hons), MIPENZ, CPEng (#242020)

Structural Engineer Studio2 Limited 111

Reviewed by:

Geoff Banks

B.E.(Hons), MIPENZ

Director

Studio2 Limited



#### **Appendix: Photos of Damage**





L6, Grid C2, brace weld L6, Grid C2, active link

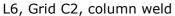






L6, Grid C2, no offset





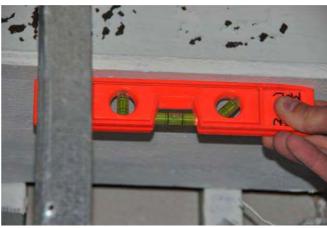
L6, Grid C2



L6, Grid C2, brace weld



L6, Grid D4(east), active link



L6, Grid D4(east), no offset



L6, Grid D4(east), brace weld



L6, Grid D4(east), column weld



L6, Grid C4(west), active link



L6, Grid C4(west), brace connection



L6, Grid C1/26 active link



L6, Grid C½6 active link



L7, Grid C2, active link



L7, Grid C2, some offset



L7, Grid C2, Lueder's lines to active link



L7, Grid C2, brace connection



L7, Grid C4(west), no offset



L7, Grid C4(west), active link



L7, Grid D4(east), no offset



L7, Grid C½6

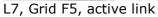


L7, Grid C1/26, minor offset



L7 floor at Grid C1/26, essentially level







L7, Grid F5, active link and brace connection



L11, Grid C2, active link



L11, Grid C2, damaged panel fixing



L11, Grid D4(east), active link



L11, Grid C4(west), active link





L11, Grid D4(west), active link



L11, grid C1/26, active link



L11, grid C½6, active link



L11, grid C1/26, active link



L11, Grid C1/26, active link



L11, Grid D4(east), active link



L11, Grid D4, column joint



L16, Grid D4(east), active link



L16, Grid D4(west) active link



L16, Grid D4(west) active link



L16, Grid C4(west), active link



L22, Grid E MRF



L22 crack to floor diaphragm (from stairwell to L22, Grid D4, active link. NE)





L22, Grid C1/2 4 and 6 active links



L2, Grid C4(west) active link



L2, Grid C4(west) active link



L2, Grid E½1, active link, permanent offset



L2, Grid E1/21, active link



L2, Grid F5, active link



L1, Grid E1, MRF



L1, Grid E1, MRF



L1, Grid F, MRF



L2 Carstacker crossbraces "unhooked"



Movement of L2 east wall



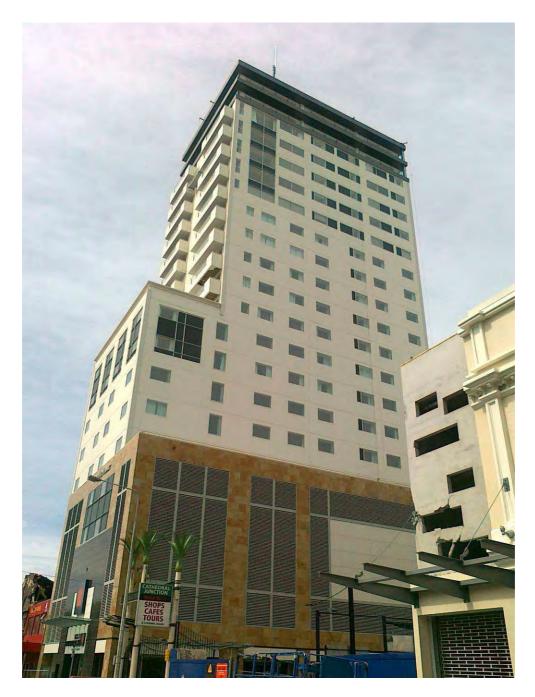
Flashings between masonry block and tower



Missing floor anchors at L2 east wall at top of ramp



Fractured carstacker top lateral restraints



Pacific Tower 2 March 2011 (some damage to north balcony soffits visible).



Inspector Initials Territorial Authority	Christchi	CPG urch City	Date Time		24 [6 [ii 3pm	Final		ing J. UNSAFE) INSPECT	1
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Structural Hazards/ Damage		nage i	Minor/None	Moderate	Severe	Comments
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	ties (eg. gas, electricity,	water)				Severe crocking to GIB board lining
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<ul> <li>Sketch (optional)</li> <li>Provide a sketch of the entire building or damage points. Indicate damage points.</li> </ul>														
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let110720-Offer fo Professional Engineering Services to Fortis Construction-ca

20 July 2011

Pacific Tower Body Corporate c/- Fortis Construction PO Box 13413 CHRISTCHURCH 8141

Attention: Ernest Duval

Email: etp@etp.co.nz

**Dear Ernest** 

#### Pacific Tower Detailed Engineering Evaluation Offer of Professional Engineering Services

Thank you for the opportunity to provide a proposal to you for professional engineering services for the above project.

We have enclosed our understanding of the scope of work, our fee proposal, proposed programme and conditions of engagement for your consideration.

#### 1. Scope of Services

We propose that the scope of our services covers the following:

- L2 Building Safety Evaluation (BSE) following 13 June, 2011 aftershocks (completed);
- Detailed Engineering Evaluation (DEE) Qualitative, in accordance with the draft SESOC/EAG
  "Detailed Engineering Evaluation Procedure", including detailed investigations of structural
  members and fixings. This would be an update and an extension of the Structex report dated 19
  May, 2011. This would also include an estimate of the peak inelastic demand in the braced
  frames and their residential capacity in conjunction with discussions with the University of
  Auckland.

#### Our deliveries will be:

- Handwritten Level 2 RAPID Building Safety Evaluation (complete);
- Detailed Engineering Evaluation (Qualitative) report.

We note the following specific exclusions to our scope of services:

- Lift shaft survey (by others)
- Exterior damage survey (by others)
- Topographical survey;
- Geotechnical engineering;
- Cost Estimating;
- Specialist advice/testing of steel, if required;
- Specific Fire engineering (by others)

#### 2. Professional Fees

We propose to carry out the above services on the basis of hourly rate plus disbursements at the following charge out rates:

Charge Out Rates

Associate (CPEng) \$260p/h Senior Engineer (CPEng) \$170p/h



Let110707 Offer of Professional Services PTBC 166 Gloucester Street

Page 2 of 2

Engineer \$120p/h

For your budgeting purposes, we estimate the following:

L2 Building Safety Evaluation \$780
 Detailed Engineering Evaluation (Qualitative) \$5000

The estimate excludes any fees or deposits that Council may levy.

A 5% charge will be added to include for disbursements such as mileage, photocopying, photography, information technology, telephone and other miscellaneous charges.

We note that any changes or variations that arise from Client requests or commencement prior to planning approvals, etc which cause additional work or rework will be an additional fee for such work and would be advised in writing, prior to the work being carried out.

We note that additional work outside our scope of service may be required but an additional fee for such work would be agreed in writing, prior to the work being carried out.

All charges quoted are exclusive of GST.

We assume that all invoices for our services will be forwarded to the same Client and address as shown on this letter. Please let us know if you would prefer the invoices to go to another address.

#### 3. Programme

We will be able to commence this work 7 days of your confirming that we are to proceed.

#### 4. Conditions of Engagement

Our terms of engagement will be in accordance with the ACENZ/IPENZ Short Form Agreement for Consultant Engagement.

A copy of which is attached.

#### 5. Conclusion

We trust we have interpreted your requirements correctly. If you wish to proceed on this basis could you please sign the attached Short Form Agreement to confirm that we are to proceed with the work either email sean.gardiner@nz.cpg-global.com or fax it 374 6516.

We thank you for this opportunity and look forward to completion of professional services that meet your expectations.

Yours sincerely

CPG

Sean Gardiner

Senior Structural Engineer

cc: Rob Young - Rob@fortisconstruction.co.nz

#### Attached

» Short Form Agreement for Consultant Engagement

	TOWER BODY CORPORATE CLIENT)						
CPG NEW ZEALAND LTD (CONSULTANT)							
Collectively referred to herein as the "Parties" and	individually as a "Party"						
PROJECT:	LOCATION:						
PACIFIC TOWER DETAILED ENGINEERING EVALUATION	166 GLOUCESTER STREET						
SCOPE & NATURE OF THE SERVICES:							
- REFER CPG LETTER DATED 7 JULY 2011							
Programme for the Services:							
- REFER CPG LETTER DATED 7 JULY 2011							
FEES & TIMING OF PAYMENTS:							
- REFER CPG LETTER DATED 7 JULY 2011 AND OVERLE	AF						
INFORMATION OR SERVICES TO BE PROVIDED BY THE CLIEN	Ι:						
NIL							
to perform the Services for the remuneration pro provision of the Short Form Model Conditions of El and any variations noted below. Once signed,	Services described above and the Consultant agrees vided above. Both Parties agree to be bound by the ngagement (overleaf), including clauses 2, 3, 9 and 10 this agreement, together with the conditions overleaf agreement previously reached between the Parties.						
VARIATIONS TO THE SHORT FORM MODEL CONDITIONS OF	ENGAGEMENT (OVERLEAF):						
NIL							
	Consultants Authorised Signatory (ies):						
NIL	CONSULTANTS AUTHORISED SIGNATORY (IES):						
NIL	CONSULTANTS AUTHORISED SIGNATORY (IES):  PRINTNAME: SEAN GARDINER						



#### SHORT FORM MODEL CONDITIONS OF ENGAGEMENT

- 1. The Consultant shall perform the Services as described in the attached documents.
- Nothing in this Agreement shall restrict, negate, modify or limit any of the Client's rights under the Consumer Guarantees
  Act 1993 where the Services acquired are of a kind ordinarily acquired for personal, domestic or household use or
  consumption and the Client is not acquiring the Services for the purpose of a business.
- 3. The Client and the Consultant agree that where all, or any of, the Services are acquired for the purposes of a business the provisions of the Consumer Guarantees Act 1993 are excluded in relation to those Services.
- 4. In providing the Services the Consultant shall exercise the degree of skill, care and diligence normally expected of a competent professional.
- 5. The Client shall provide to the Consultant, free of cost, as soon as practicable following any request for information, all information in his or her power to obtain which may relate to the Services. The Consultant shall not, without the Client's prior consent, use information provided by the Client for purposes unrelated to the Services. In providing the information to the Consultant, the Client shall ensure compliance with the Copyright Act 1994 and shall identify any proprietary rights that any other person may have in any information provided.
- 6. The Client may order variations to the Services in writing or may request the Consultant to submit proposals for variation to the Services. Where the Consultant considers a direction from the Client or any other circumstance is a Variation the Consultant shall notify the Client as soon as practicable.
- 7. The Client shall pay the Consultant for the Services the fees and expenses at the times and in the manner set out in the attached documents. Where this Agreement has been entered by an agent (or a person purporting to act as agent) on behalf of the Client, the agent and Client shall be jointly and severally liable for payment of all fees and expenses due to the Consultant under this Agreement.
- 8. All amounts payable by the Client shall be paid within twenty (20) working days of the relevant invoice being mailed to the Client. Late payment shall constitute a default, and the Client shall pay default interest on overdue amounts from the date payment falls due to the date of payment at the rate of the Consultant's overdraft rate plus 2% and in addition the costs of any actions taken by the Consultant to recover the debt.
- 9. Where Services are carried out on a time charge basis, the Consultant may purchase such incidental goods and/or Services as are reasonably required for the Consultant to perform the Services. The cost of obtaining such incidental goods and/or Services shall be payable by the Client. The Consultant shall maintain records which clearly identify time and expenses incurred.
- 10. Where the Consultant breaches this Agreement, the Consultant is liable to the Client for reasonably foreseeable claims, damages, liabilities), losses or expenses caused directly by the breach. The Consultant shall not be liable to the Client under this Agreement for the Client's indirect, consequential or special loss, or loss of profit, however arising, whether under contract, in fort or otherwise.
- 11. The maximum aggregate amount payable, whether in contract, tort or otherwise, in relation to claims, damages, liabilities, losses or expenses, shall be five times the fee (exclusive of GST and disbursements) with a maximum limit of \$NZ250,000.
- Neither Party shall be liable for any loss or damage occurring after a period of six years from the date on which the Services were completed.
- 13. The Consultant acknowledges that the Consultant currently holds a policy of Professional Indemnity insurance for the amount of liability under clause 11. The Consultant undertakes to use all reasonable endeavours to maintain a similar policy of insurance for six years after the completion of the Services.
- 14. If either Party is found liable to the other (whether in contract, tort or otherwise), and the claiming Party and/or a Third Party has contributed to the loss or damage, the liable Party shall only be liable to the proportional extent of its own contribution.
- 15. The Consultant shall retain intellectual property/copyright in all drawings, specifications and other documents prepared by the Consultant. The Client shall be entitled to use them or copy them only for the works to which the Services relate and the purpose for which they are intended. The ownership of data and factual information collected by the Consultant and paid for by the Client shall, after payment by the Client, lie with the Client. The Client may reproduce drawings, specifications and other documents in which the Consultant has copyright, as reasonably required in connection with the project but not otherwise. The Client shall have no right to use any of these documents where any or all of the fees and expenses remain payable to the Consultant.
- 16. The Consultant has not and will not assume any obligation as the Client's Agent or otherwise which may be imposed upon the Client from time to time pursuant to the Health and Safety in Employment Act 1992 ("the Act") arising out of this engagement. The Consultant and Client agree that in terms of the Act, the Consultant will not be the person who controls the place of work.
- 17. The Client may suspend all or part of the Services by notice to the Consultant who shall immediately make arrangements to stop the Services and minimise further expenditure. The Client and the Consultant may (in the event the other Party is in material default) terminate the Agreement by notice to the other Party. Suspension or termination shall not prejudice or affect the accrued rights or claims and liabilities of the Parties.
- 18. The Parties shall attempt in good faith to settle any dispute by mediation.
  - 19. This Agreement is governed by the New Zealand law, the New Zealand courts have jurisdiction in respect of this Agreement, and all amounts are payable in New Zealand dollars.

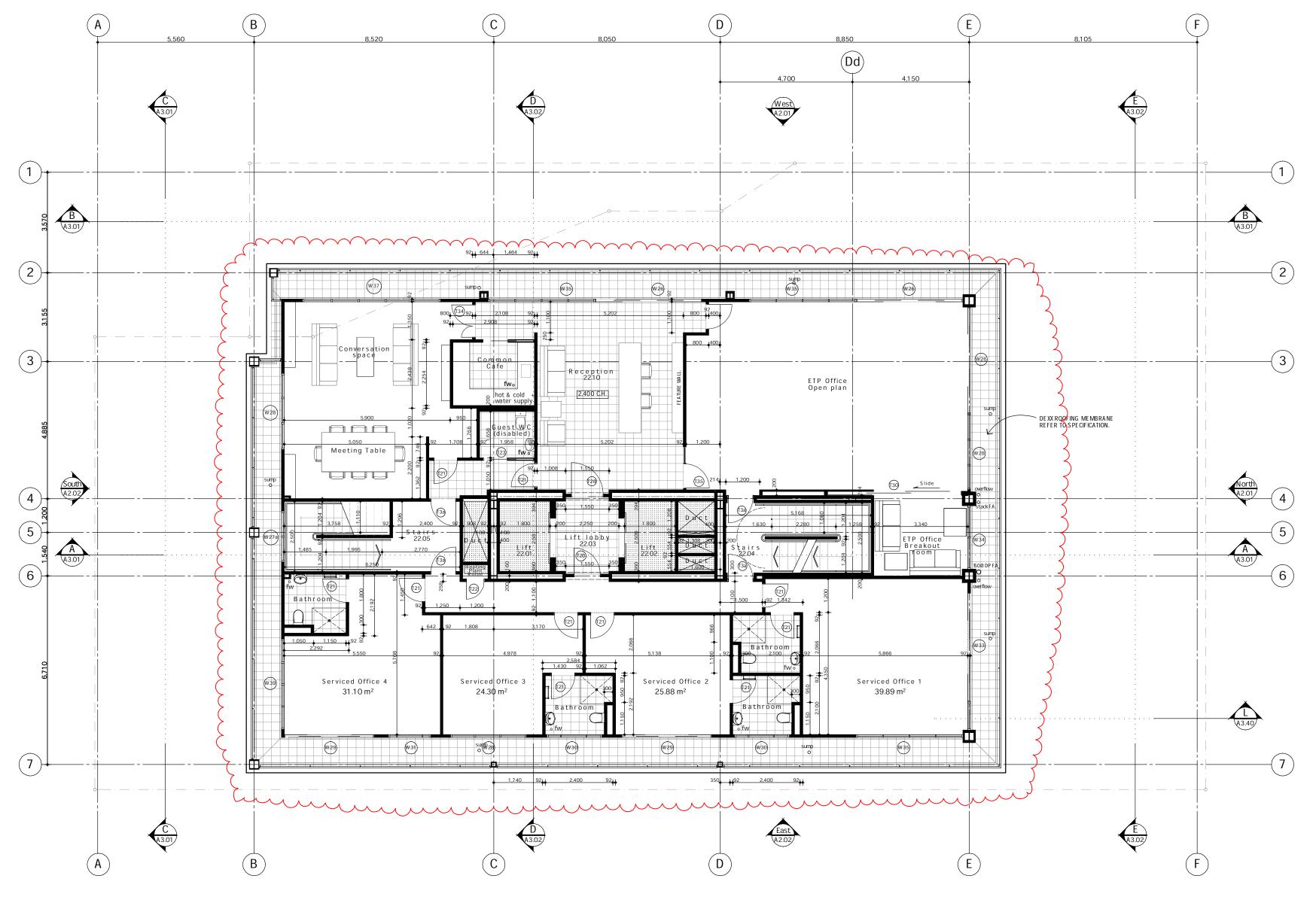
March 2010 Page 2



### Site Inspection Record Sheet



Job NameP	active Tower	File No					
Job Location	166 Glancester St.	Job No					
		Ref No					
Inspection Type	EQ repairs.						
Inspection Date	9 / 7 / [1 Time11:00	Page					
Weather Fiv							
Inspection Undertake		Contact No 0212432553					
		Contact No Oct 0430533					
Inspection Undertake	en With Lift technicians.						
Other							
Item No.	Observatio	ns / Notes					
	Enspected Northern lift shaft from top of lift						
	Cax. L2 - L8.						
	Further minor bosing of confirm new permonent	1. It short to East. Pleas					
	contrin new permonent	offsets in both directions					
	Comment was requested an	being able to noteh the					
	Comment was requested an flanges of a steel brace	to allow hate further					
	lift door movement.						
	the third	elevation					
	plan	elevation					
	e.g. lift shaft	Notching passible?					
	Preliminery advice, including a	discussions with the University					
	of Conterbury, suggests the be altered; and future tollo	rance should be abbition by					
*	relocating the lift gear.	The sound of					
	7	1 90.					
	Inspector's Signature	Len Jarlin					



LEVEL 22 - EXECUTIVE OFFICES

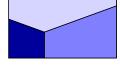
#### Mh 01/07/09 Dimensions added. Mg 17/06/09 Add hot & cold water supply + floor wastes. Mg1 15/05/09 ETP Executive Offices with 4 x services offices. Mf 09/10/08 Level 22 to match Level 21 Apartment Me 14/08/08 Windows Moved Board Room Altered Md 16/06/08 Dimensions added. Mc 29/04/08 Balcony handrail and columns revised. Mb 14/03/08 Door numbers revised. Ma 23/10/07 duct change. M 13/09/07 Client Approval L 15/08/07 Client Approval K 02/08/07 Client Approval J 27/07/07 Client Approval H 23/07/07 Client Approval G 13/07/07 Client Approval F 06/07/07 Client Approval E 03/07/07 Client Approval D 22/06/07 Client Approval Client Approval 2 A 29/05/07 Client Approval S | - 27/04/07 | Building Consent Issue # DATE DESCRIPTION

Mj 02/07/09 Relabelled to Executive Offices

# FOLEYDESIGN architecture

Fax +64 3 377 5667 E mail info@ foleydesign.co.nz

Tel +64 3 377 5666



30 Southwark Street P 0 Box 22166 Christchurch, New Zealand

THE COPYRIGHT IN THE DESIGN & DRAWINGS IS OWNED BY FOLEY DESIGN LIMITED & AMC CONSTRUCTION LIMITED

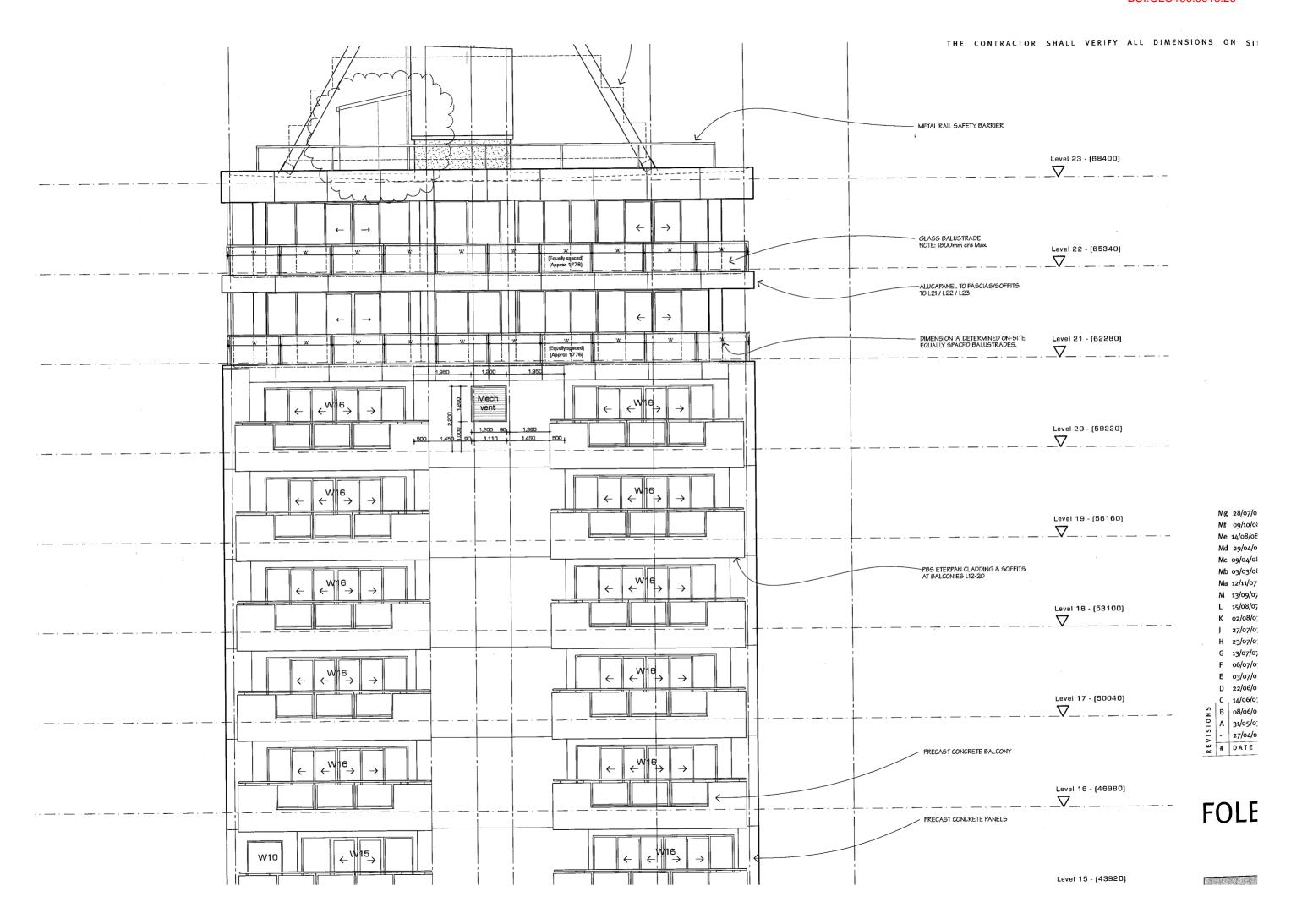
## PROJECT TITLE C1 TOWER

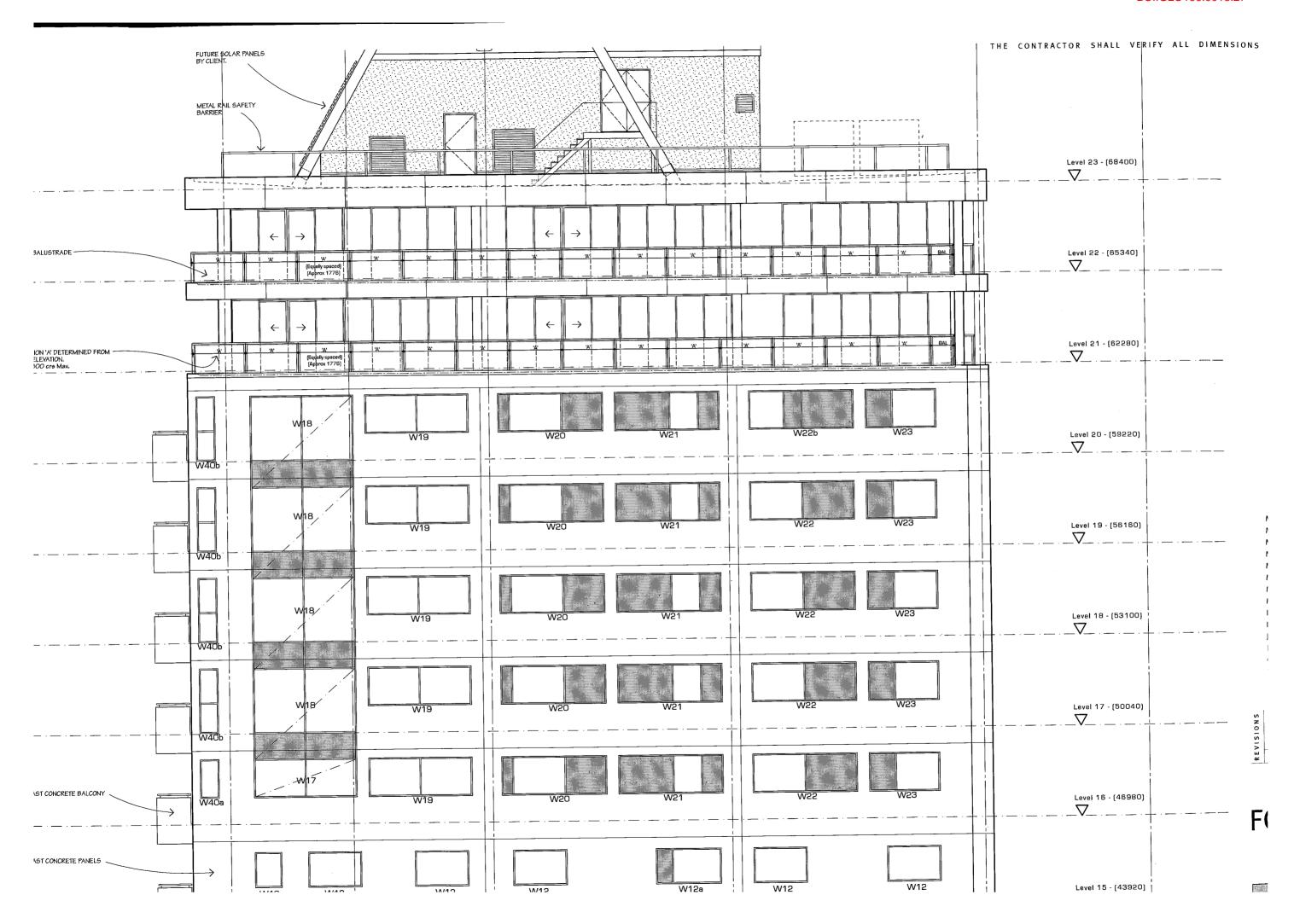
FILE NUMBER 1403

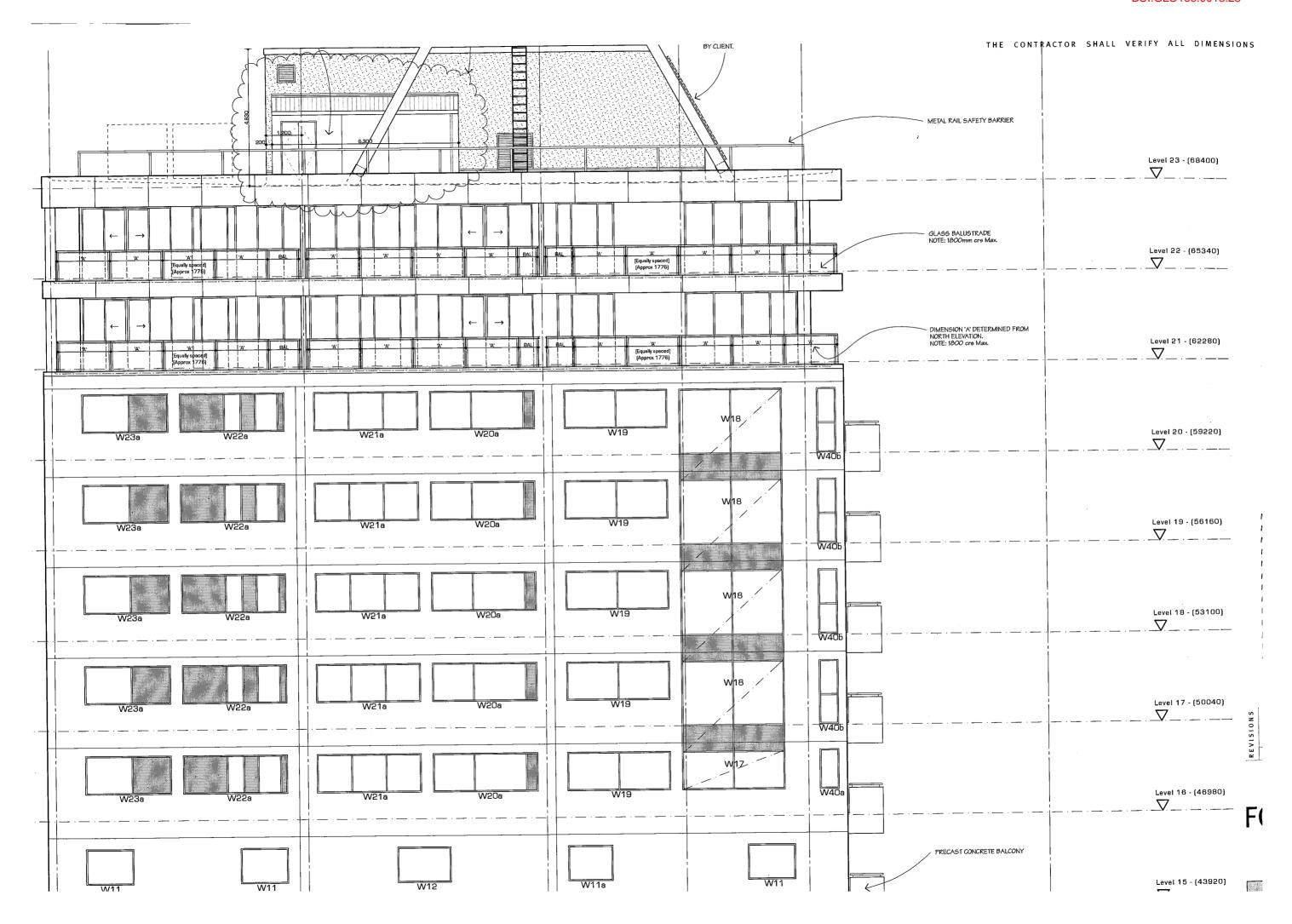
DRAWING TITLE

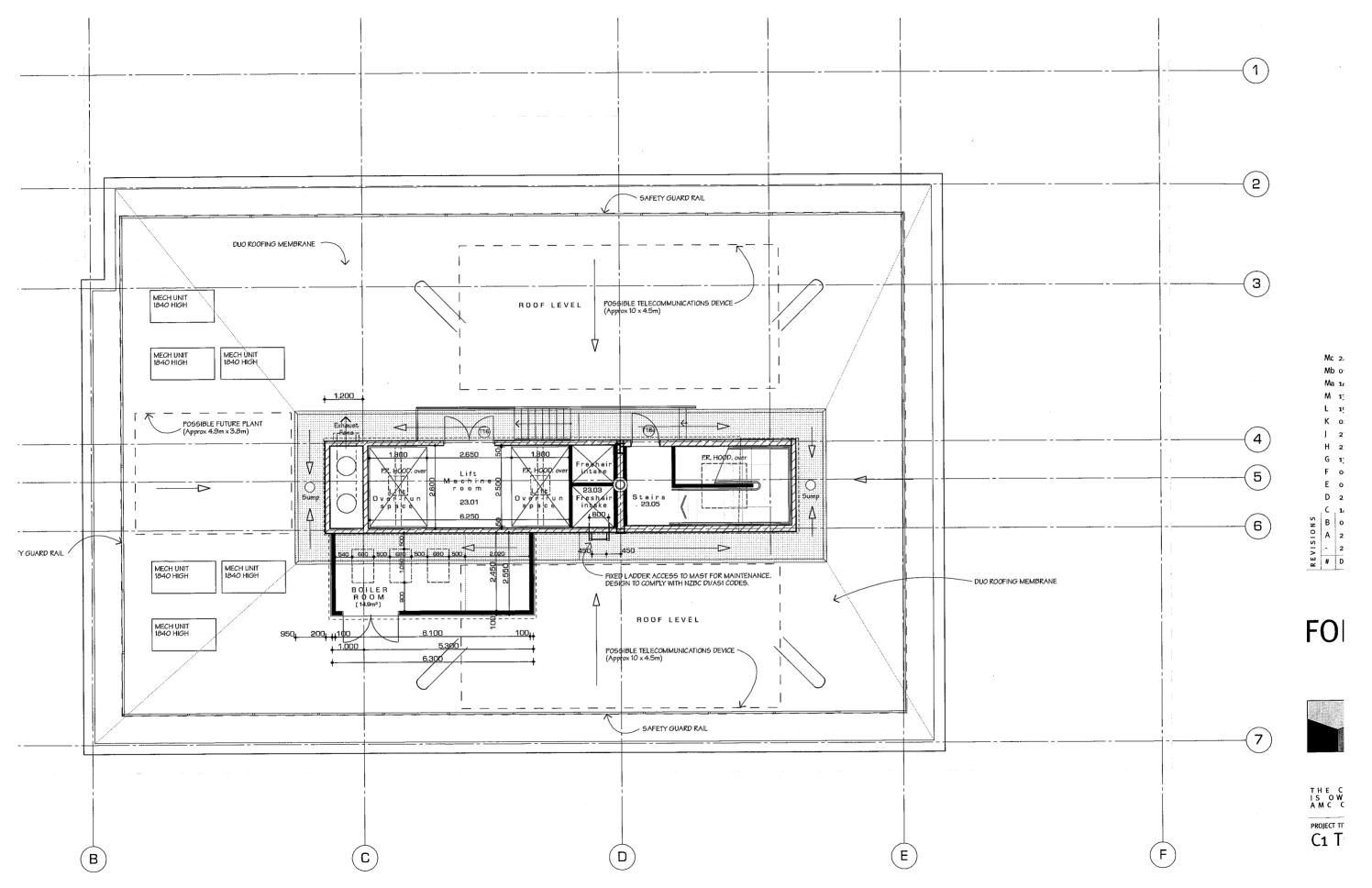
LEVEL 22

DRAWING NUMB	A1.22	REVISION	Mi
SCALE	1: 100 @ A2	DATE	02 July 2009
DESIGNED BY:	Rob Campbell	DRAWN BY:	Steve Foote









ROOFING LEVEL 23 LIFT PLANT / OVER-RUN Scale 1:100

DRAWING T

report		Date:	17 May 2007	st	ructex
Project:	C1 Tower	Ву:	Sean Gardiner	Ref:	10053
Title	Building Analysis Assum	ptions & Design Proc	edures		

#### **ETABS Model**

- Our calculated total building seismic weight (G+0.3Q) is 63340 kN. This has an allowance for additional floor at levels 3 and 5, areas of plant, a 50 kN mass at top of building, and 120mm thick precast panels up to the underside of level 20. I have attached our load take for your information.
- Designed to AS/NZS 1170 response spectrum for Christchurch, category D soils, (base shear scaled to 100% equivalent static base shear).
- We have assumed rigid diaphragms.
- The foundations have been modelled as pin-based (to model potential hinges), with vertical springs to model foundation flexibility. Following discussions with Geotech Consulting, we have assumed a deflection of +/- 10mm under a ULS earthquake would be reasonable.
- Fundamental building periods were determined as T=3.68 sec (long direction) and T=3.04 sec (short direction).
- P-delta effects were included in the analysis based on AS/NZS 1170. IE. we assessed the
  elastic deflection and found the required forces at each level to resist the displaced weight of
  each level. These forces were added as a static load case to the model response cases
  (Absolute Values). We did not use the ETABS P-delta automatic module.

#### Design

- Design of EBFs based on NZS 3404 and HERA Report R4-76, as well as parts of proposed amendment 2 to NZS 3404 and discussions with HERA and SCNZ.
- The maximum design actions on members were based on the over-strength of the actual links or  $E_{max}$  (typically corresponding to  $\mu = 1.25$  earthquake as  $\mu_{act} > 1.5$ , except for grid 2 (where  $\mu_{act} < 1.5$ ,  $E_{max}$  based on  $\mu = 1.0$  levels of load).
- The frames were designed to be limited ductile ie.  $\mu = 3.0$  in the short director and  $\mu = 2.4$  in the long direction (based on minimum base shear coefficient = 0.031).
- The active links were sized to resist  $\mu$  (design) actions as well as the gravity shear component through the links (which became significant for D-braced frames). The link length was sized less than 1.6 Ms/Vv to ensure primarily shear yielding. We found the moment in the link due to gravity plus the moment due to the brace caused the link to approach yield in flexure as well as shear, and this was acceptable to HERA. The link rotation was limited to 0.09 radians as per NZS 3404, and stiffened appropriately. Restraint is provided by the floor slab typically (except grid 1 frame at levels 3, 4 and 5 where it is provided by a 327 HCC beam on its side).



- The braces and collector beams were designed for the over-strength of the link, or E<sub>max</sub>, plus the gravity component in the member, plus the gravity component of shear going through the link where appropriate. This axial force was combined with the bending in the member due to the earthquake and gravity loads for the design, (ie. combined actions).
- The columns design was generally based on  $E_{max}$  (as the dynamic magnification factor in conjunction with the over-strength loads, was generally much greater) in conjunction with local gravity loads plus gravity loads from the active link where appropriate. The columns C2 and D2 are unrestrained between levels 2 and 6 in the strong axis direction. The columns on C2, C6, D2 and D6 were subjected to bi-axial bending, even in perpendicular earthquakes, as well as axial load. They were also subjected to concurrent actions of 100% in one direction and 30% in the perpendicular direction (due to the low actual ductility) in accordance with AS/NZS 1170, following discussions with HERA. The columns (and beams and braces, where appropriate) were designed to the alternative design provisions of NZS 3404 with  $\beta_m$ =0. Columns C2 and D2 are concrete encased at level 1.

We used the axial load limits from the proposed amendment 2 to NZS 3404 (end yielding criteria) for the design (refer cl.12.8.3.1), as an alternative to the current NZS 3404 requirements.

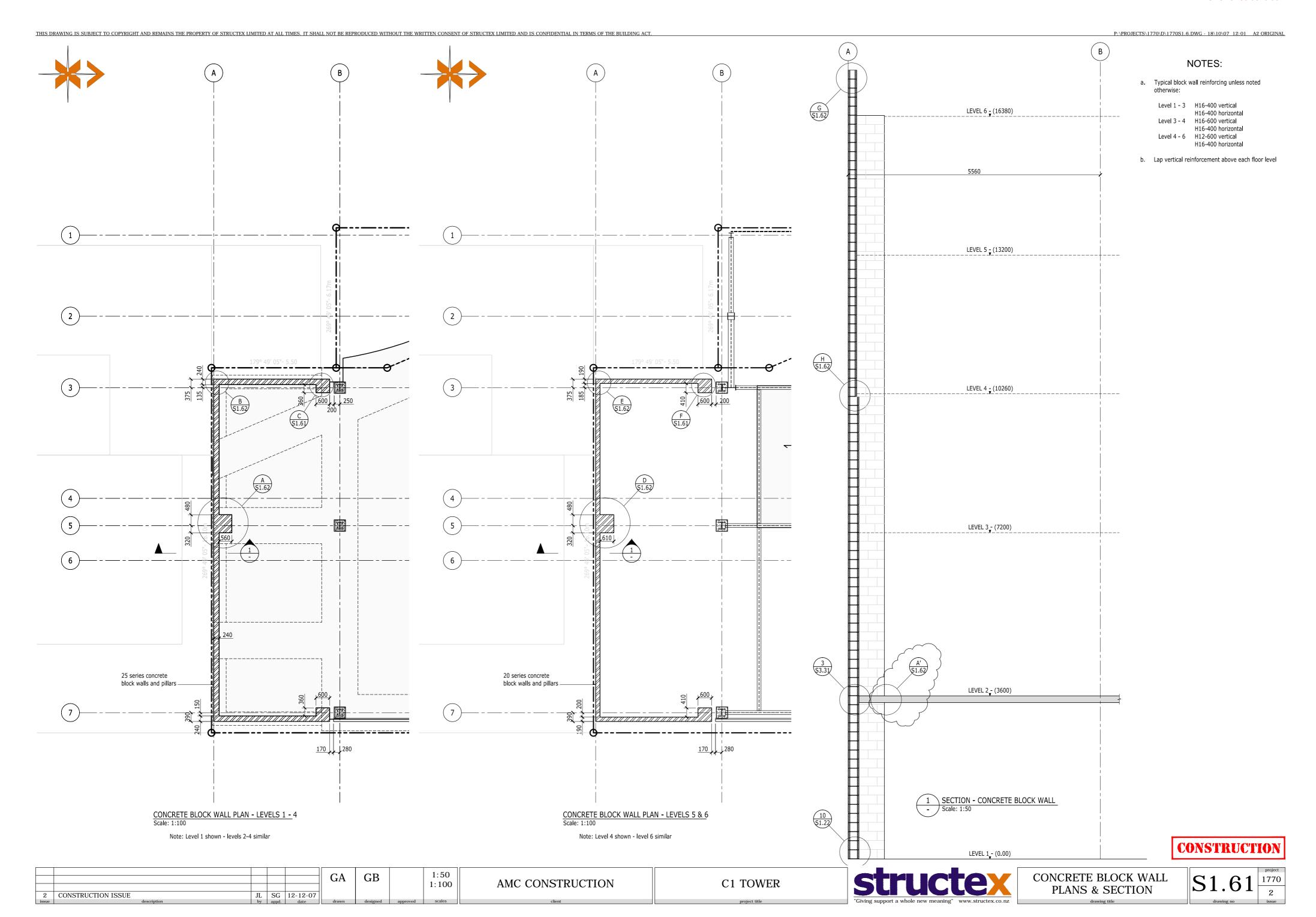
The columns were also designed for the moment induced from noding eccentricity where the beam-brace centrelines lined up with the column flanges, (where the beam and brace came into the web the centrelines line up and there is no noding eccentricity).

- The studs have generally been designed to take  $E_{max}$  loads ( $\mu$ =1.25) with a capacity of 62.6 kN/stud (19 dia x 125 long), from ComFlor literature).
- The connections have generally been designed for the greater of minimum design actions from NZS 3404 or  $E_{\text{max}}$  plus gravity loads. The gravity column splices have full contact bearing and therefore nominal connections to resist minimum design actions. The seismic columns splices have full contact bearing and have been design for earthquake tension as well as bi-axial bending and shear. The braces and beams have been designed without full contact bearing, as full tension splices. The gussets/bolts and connections take the components of shear in each direction. Following discussions with HERA, as the noding eccentricity was taken account of in the column design the gussets did not require design to the uniform force method.
- The fillet welds in the Steltech sections, between the flanges and web, were designed to take the maximum shear flow actions (based on maximum shear, or change in moment) in the columns. Where a Steltech section is required as a beam/active link, the flanges have been fully welded to the web with complete penetration butt welds. I have attached useful Steltech section tables for your information.

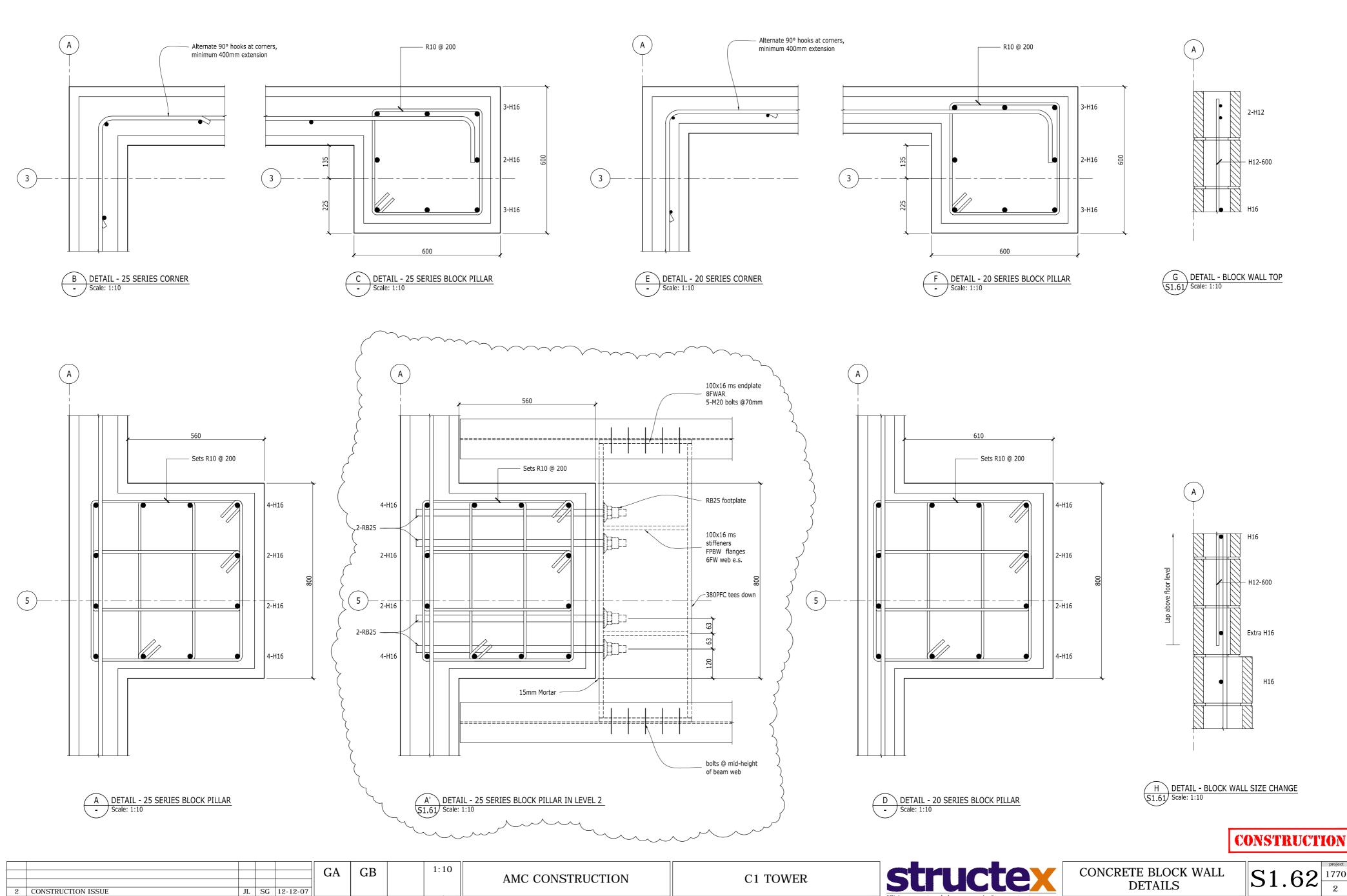
**Sean Gardiner** 

Engineer





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JL SG 12-12-07 by appd. date

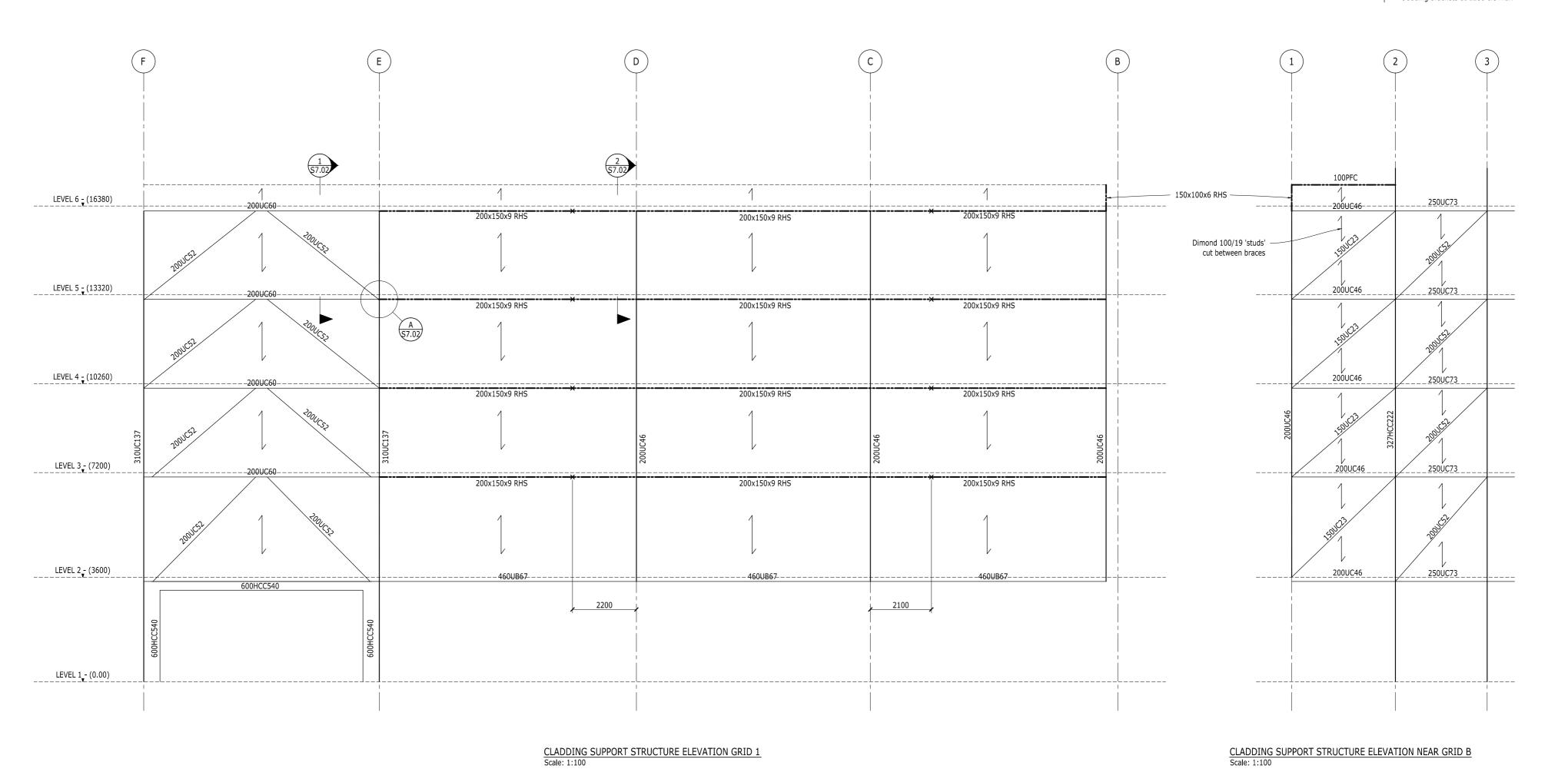
2 CONSTRUCTION ISSUE

#### NOTES:

x Beam Splice position - refer sheet S7.02 for detail

Dimond 100/19 'studs' to suit cladding brackets at 1200 crs max

Dimond 100/19 canterlever 'studs' to suit cladding brackets at 1200 crs max



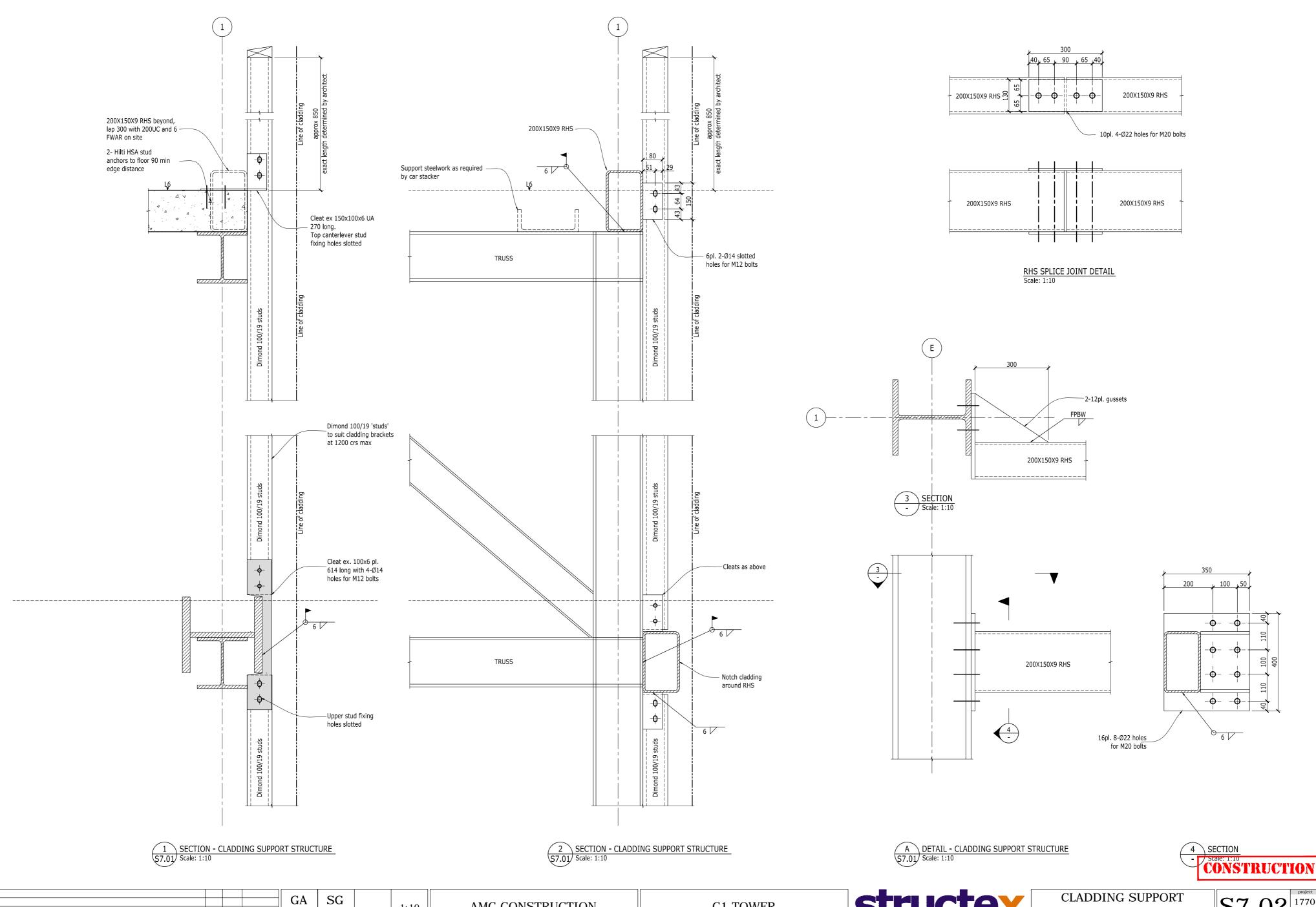
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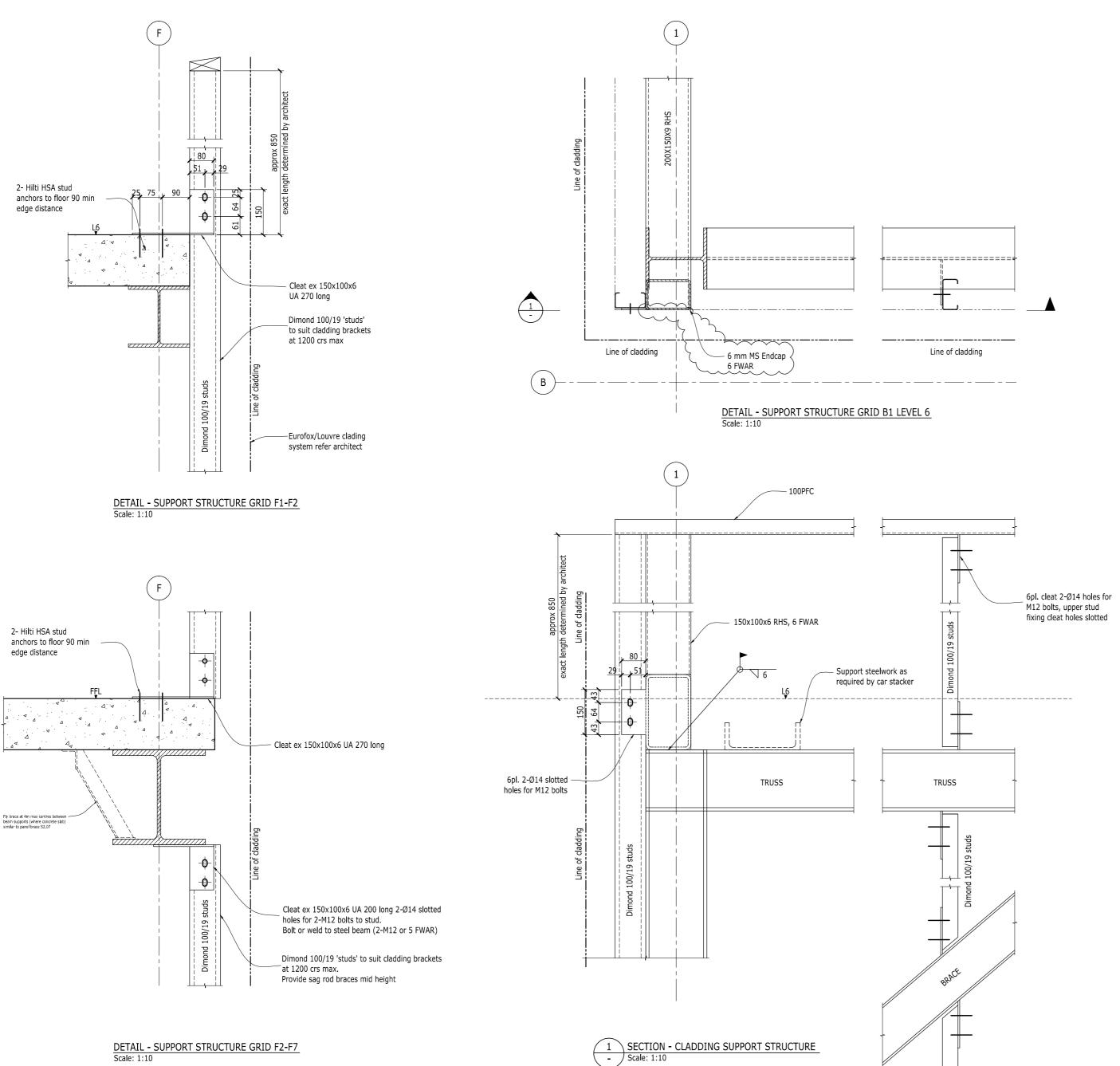


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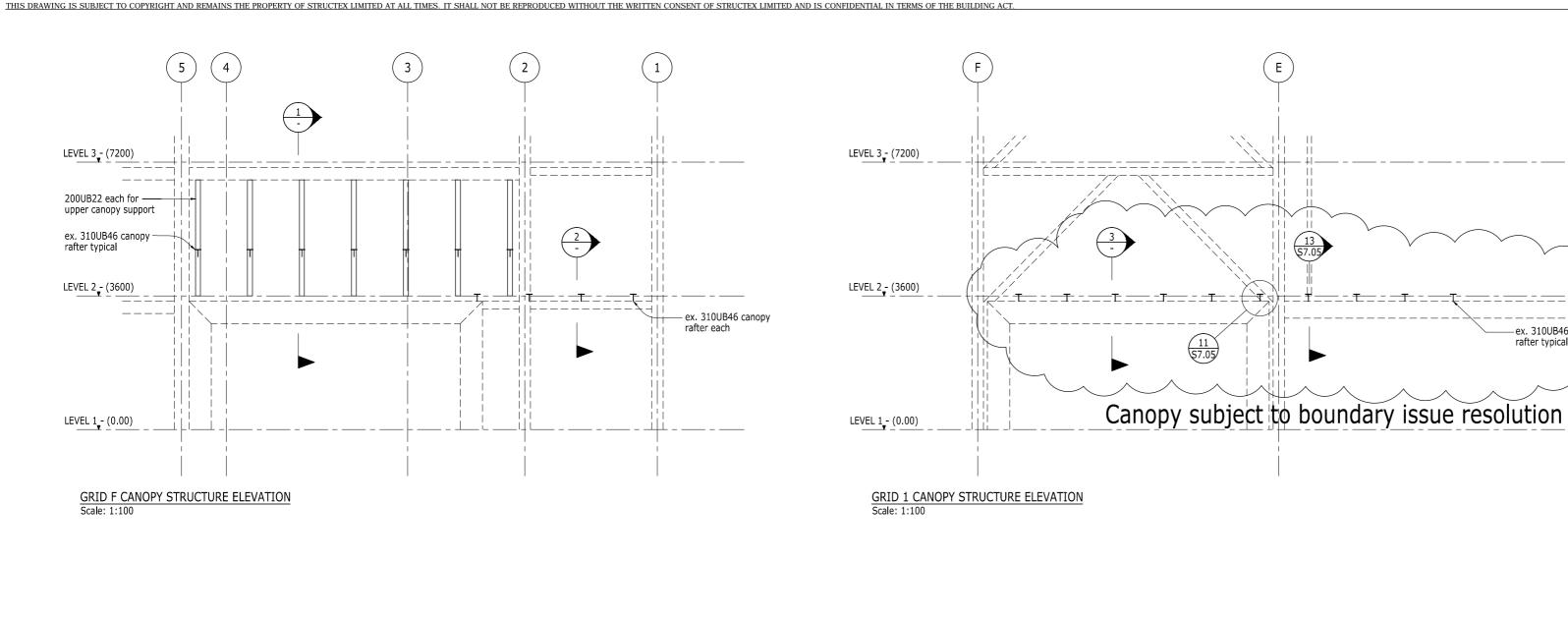
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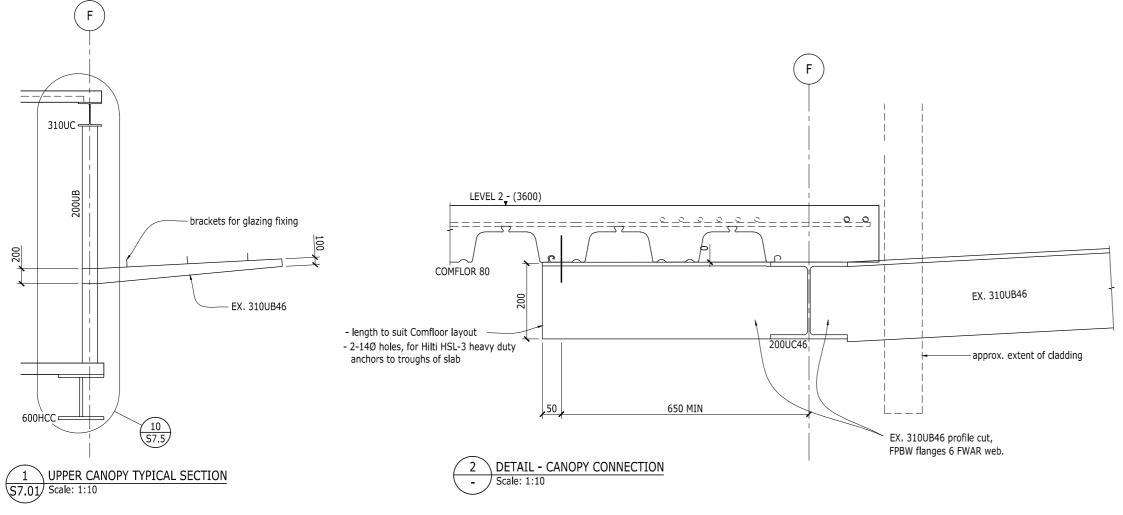
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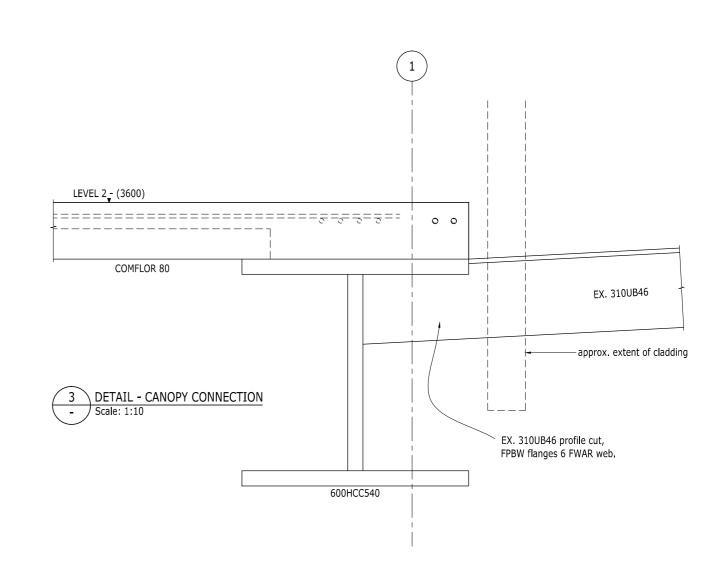




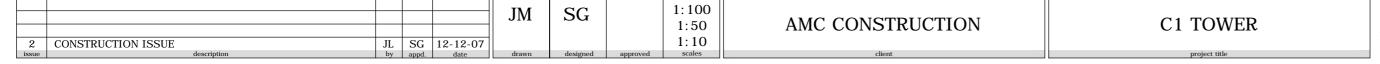
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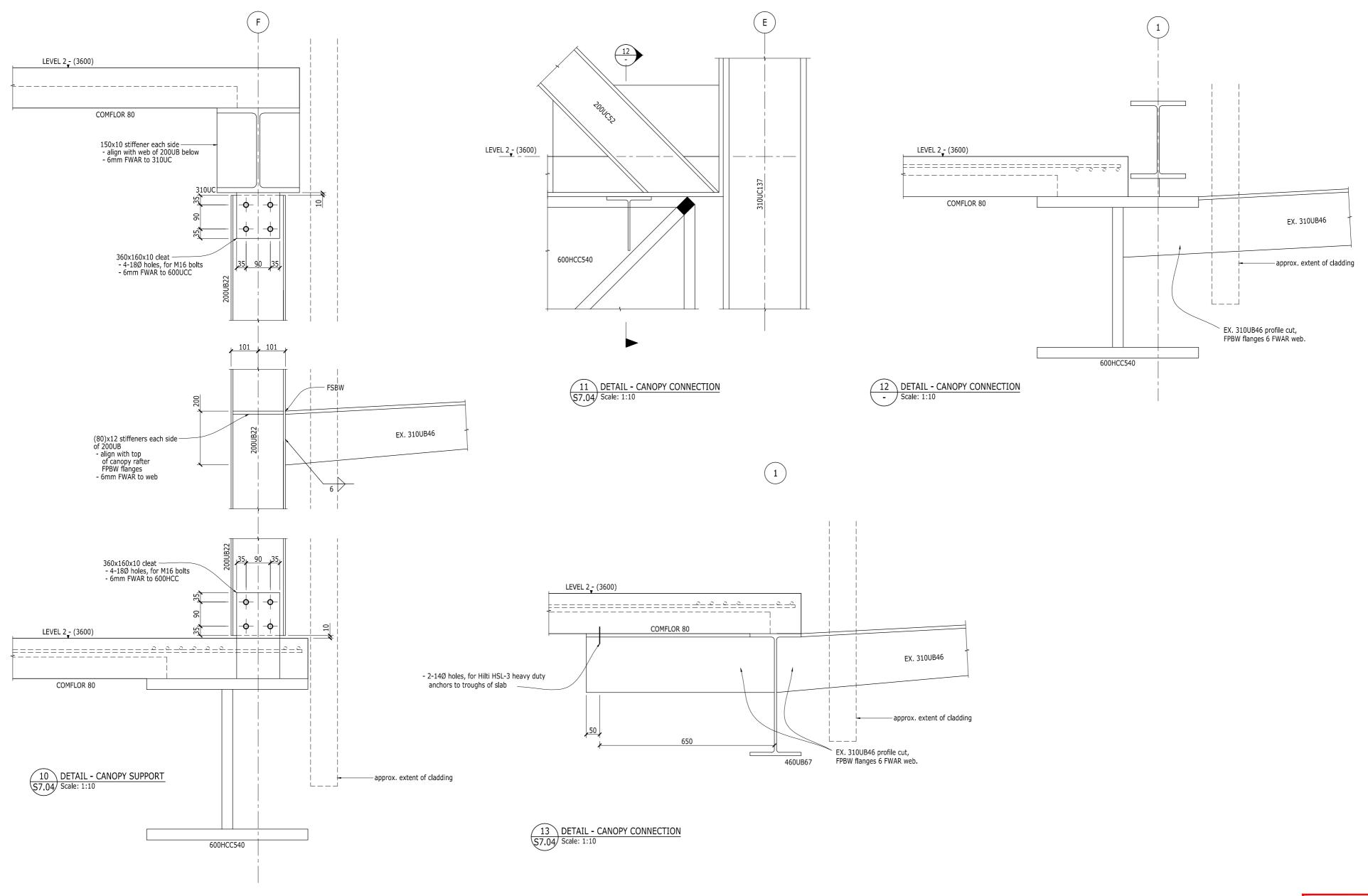
ex. 310UB46 canopy rafter typical







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CONSTRUCTION

SG 1:100 JM C1 TOWER AMC CONSTRUCTION 1:50 JL SG 12-12-07 by appd. date 1:10 2 CONSTRUCTION ISSUE





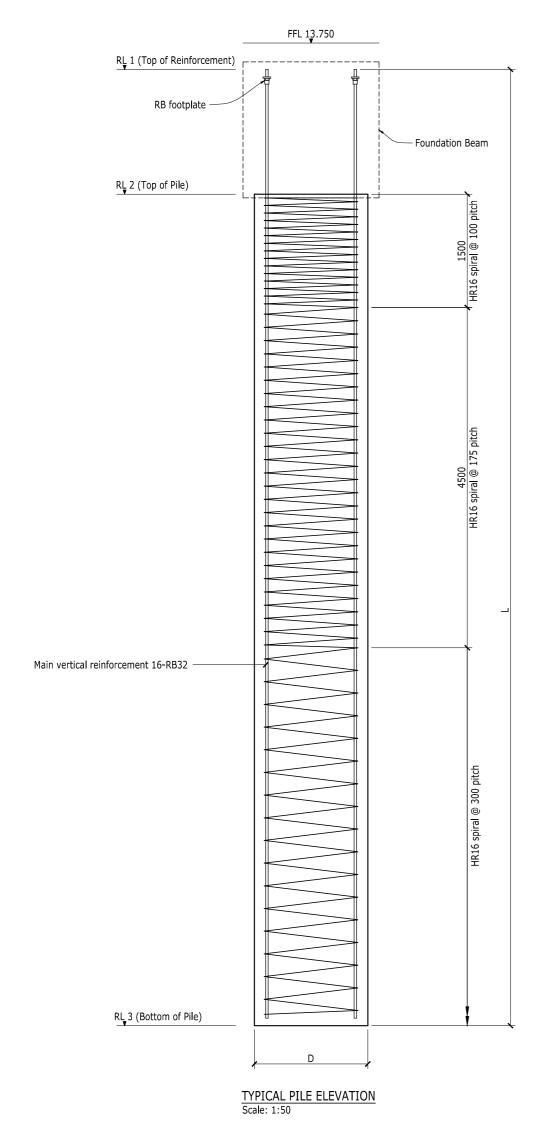
P:\PROJECTS\1770\D\1770S1.0.DWG - 22\05\07 11:28 A2 ORIGINAL THIS DRAWING IS SUBJECT TO COPYRIGHT AND REMAINS THE PROPERTY OF STRUCTEX LIMITED AT ALL TIMES. IT SHALL NOT BE REPRODUCED WITHOUT THE WRITTEN CONSENT OF STRUCTEX LIMITED AND IS CONFIDENTIAL IN TERMS OF THE BUILDING ACT. <u>LEGEND</u> E New Ø 1500 Concrete pile New Ø 900 Helix Screw pile with 830 (Dd) kN ULS Tension Capacity - 12m depth < 1500 4700 4150 New Ø 900 Helix Screw pile raking to 12m depth PILE SETOUT 1500 1500 1500 300 SCREW PILE SETOUT Tension Anchor 1000kN Ultimate Capacity (1)Proposed Test Pile SCREW PILE SETOUT 1200 400 Deep Foundation 2 900 Deep Foundation 1800 Deep Foundation Beam 3 21 <del>|</del> 23 4 33 \* 28 (6 )-1650 1100 1300 10 19 (29) PILE SETOUT PILE SETOU PILE SETOUT , 800 1500

C	0	N	S	1	N	T
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0A 0	CONSTRUCTION UPDATE CL FOR CONSENT GA	GB GB	28-06-07 12-03-07	GA	GB		1:100	AMC CONSTRUCTION	C1 TOWER	struc	
issue	description by	appd.	date	drawn	designed	approved	scales	client	project title	"Giving support a whole new meaning	g" www.structex.co.n



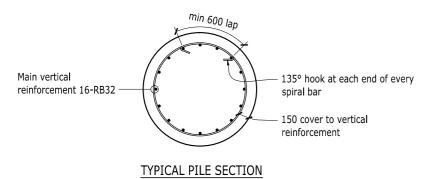


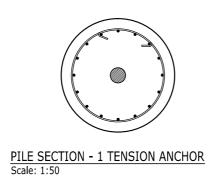


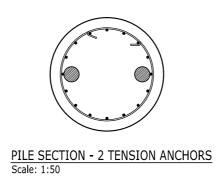
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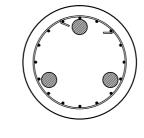
Pile No	Diameter (D) (mm)	Length (L) (mm)	No of Tension Anchors	RL 1	RL 2	RL 3	Notes
1	1500	8000	-	13.400	12.650	3.750	
2	1500	8000	-	13.400	12.650	5.400	
3	1500	12000	-	13.400	12.650	1.400	
4	1500	12000	-	13.400	12.650	1.400	
5	1500	12000	-	13.400	12.650	1.400	
6	1500	12000	1	13.400	11.750	1.400	
7	1500	8000	-	13.400	11.750	5.400	
8	1500	12000	2	13.400	11.750	1.400	
9	1500	12000	2	13.400	11.750	1.400	
10	1500	12000	-	13.400	12.650	1.400	
11	1500	12000	1	13.400	11.750	1.400	
12	1500	12000	1	13.400	11.750	1.400	
13	1500	8000	-	13.400	11.750	5.400	
14	1500	12000	2	13.400	11.750	1.400	
15	1500	12000	-	13.400	11.750	1.400	
16	1500	12000	1	13.400	11.750	1.400	
17	1500	8000	2	13.400	11.750	5.400	
18	1500	8000	2	13.400	11.750	5.400	
19	1500	12000	-	13.400	12.650	1.400	
20	1500	12000	-	13.400	11.750	1.400	
21	1500	12000	1	13.400	11.750	1.400	
22	1500	8000	2	13.400	11.750	5.400	
23	1500	8000	-	13.400	12.650	5.400	
24	1500	8000	-	13.400	12.650	5.400	
25	1500	8000	-	13.400	11.750	5.400	
26	1500	8000	-	13.400	11.750	5.400	
27	1500	12000	-	13.400	12.650	1.400	
28	1500	12000	-	13.400	12.650	1.400	
29	1500	8000	-	13.400	12.650	3.750	
30	1500	8000	-	13.400	11.750	5.400	
31	1500	8000	3	13.400	11.750	5.400	
32	1500	8000	-	13.400	11.750	5.400	
33	1500	12000	-	13.400	11.750	1.400	
34	1500	12000	3	13.400	12.650	1.400	











PILE SECTION - 3 TENSION ANCHORS
Scale: 1:50

NOTE:

Tension anchors shall be designed and installed by the contractor to provide a design tension load at a ULS loading of 1000kN

Refer to the architect's site level details for the level datum

structey

CONSENT

0A	PILES 1 & 29 DEPTH REVISED	GA	GB	21-03-07	
0	FOR CONSENT	GA	GB	12-03-07	
issue	description	by	appd.	date	

GA GB

1:50 AMC

AMC CONSTRUCTION

C1 TOWER

PILE DETAILS

S1.02 1770 OA

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FOUNDATION BEAM PLAN



CONSENT

D PRELIMINARY GA GB 08-03-07 BG GA GB GA GB 07-03-07 C PRELIMINARY AMC CONSTRUCTION 1:100 GA GB 28-02-07
GA GB 05-02-07
by appd. date B PRELIMINARY A PRELIMINARY

CL GB 18-04-07

CL GB 29-03-07

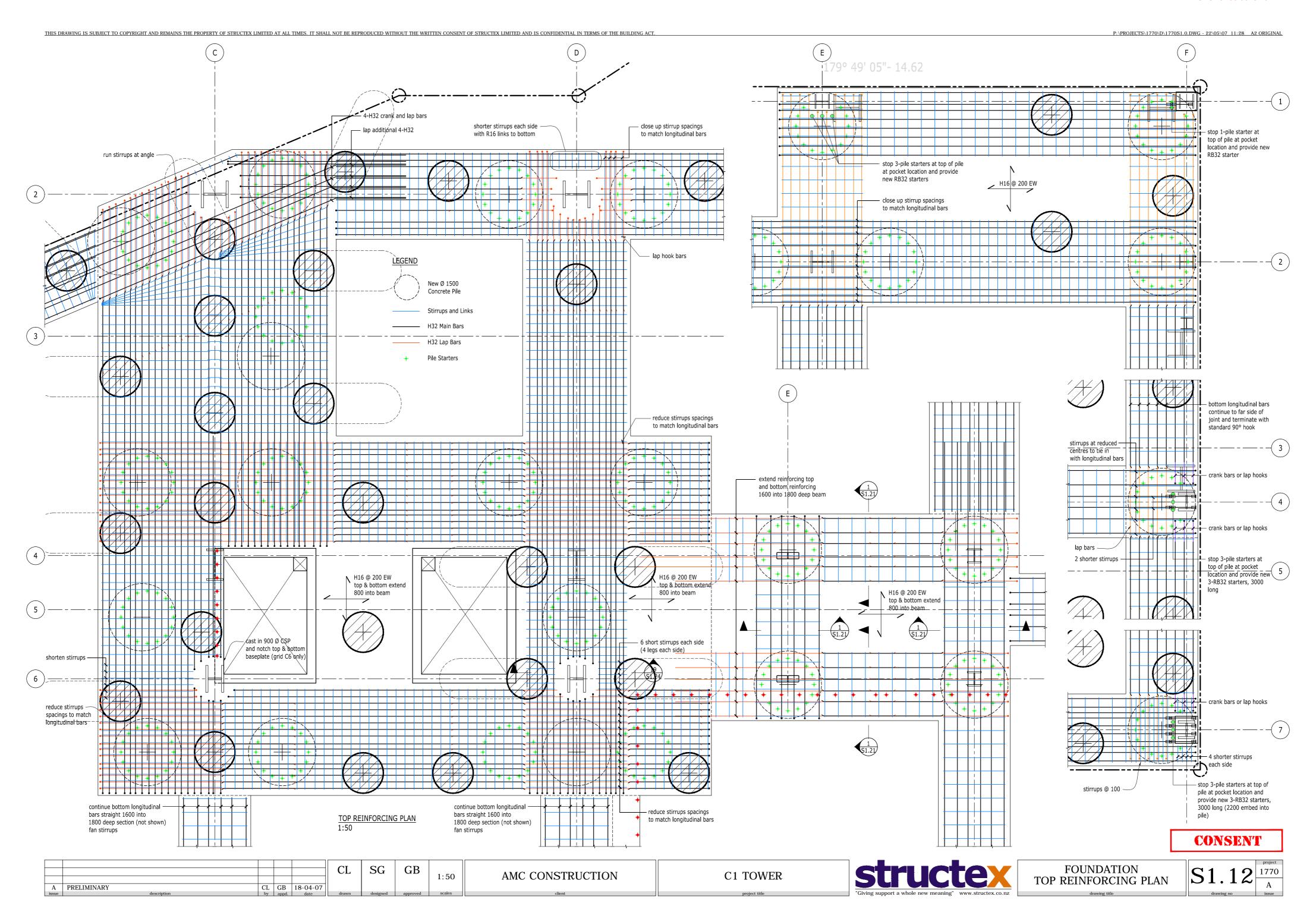
CL GB 15-03-07

E PRELIMINARY

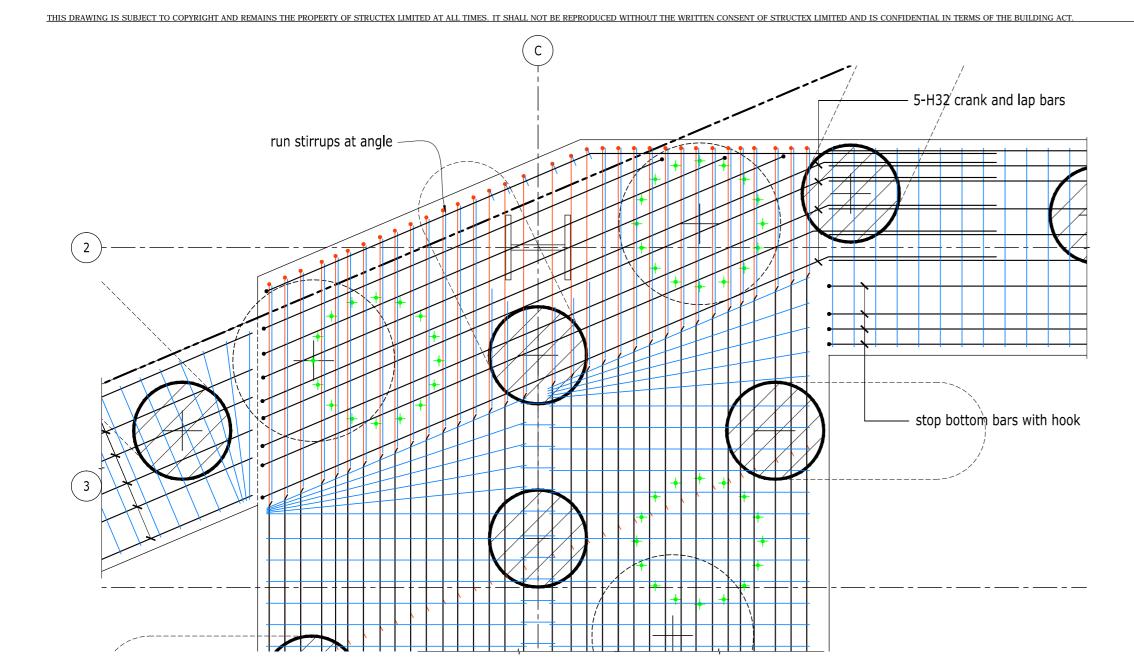
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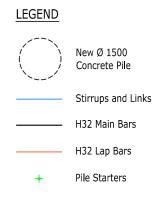
0 FOR CONSENT

C1 TOWER



A2 ORIGINAL



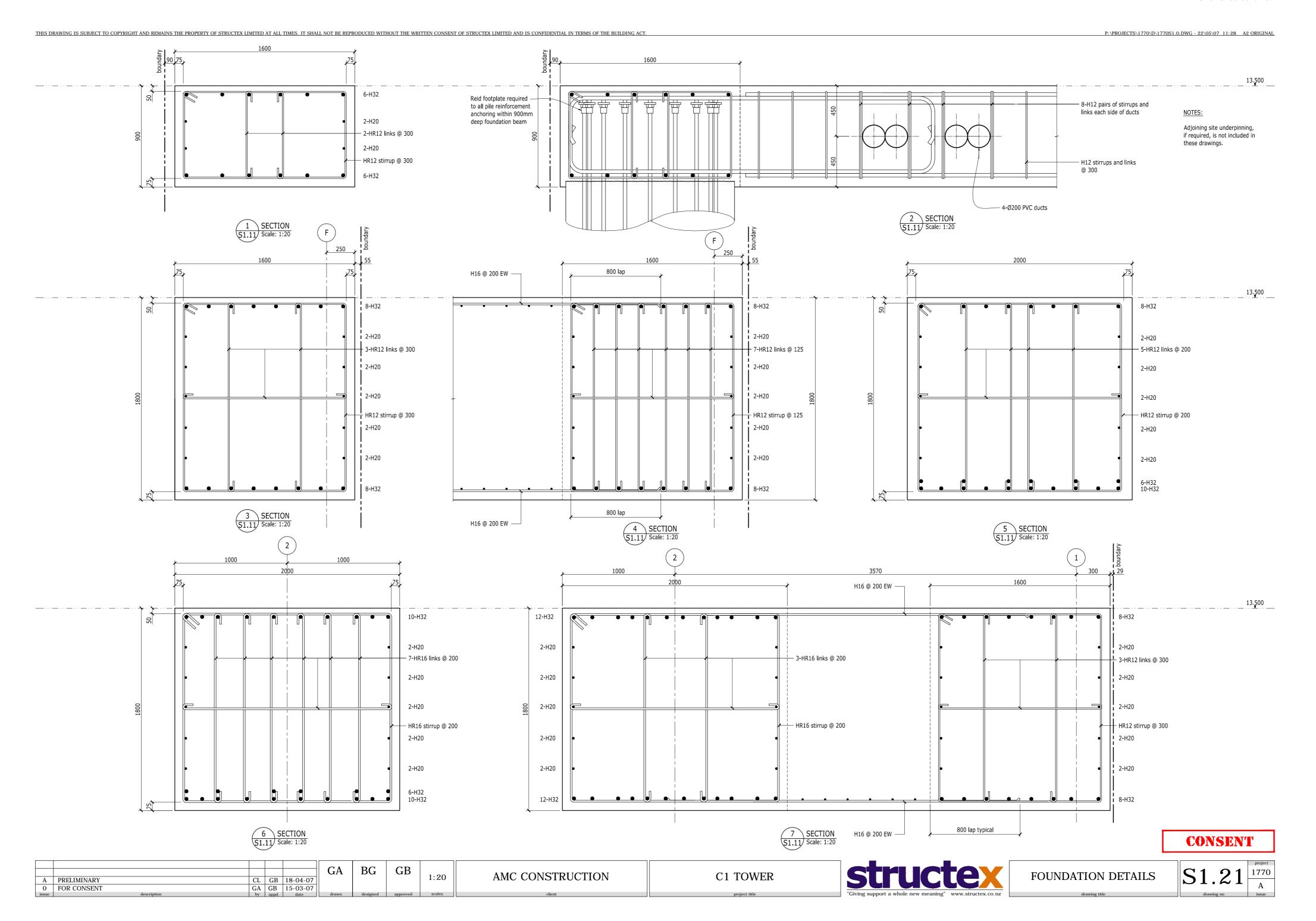


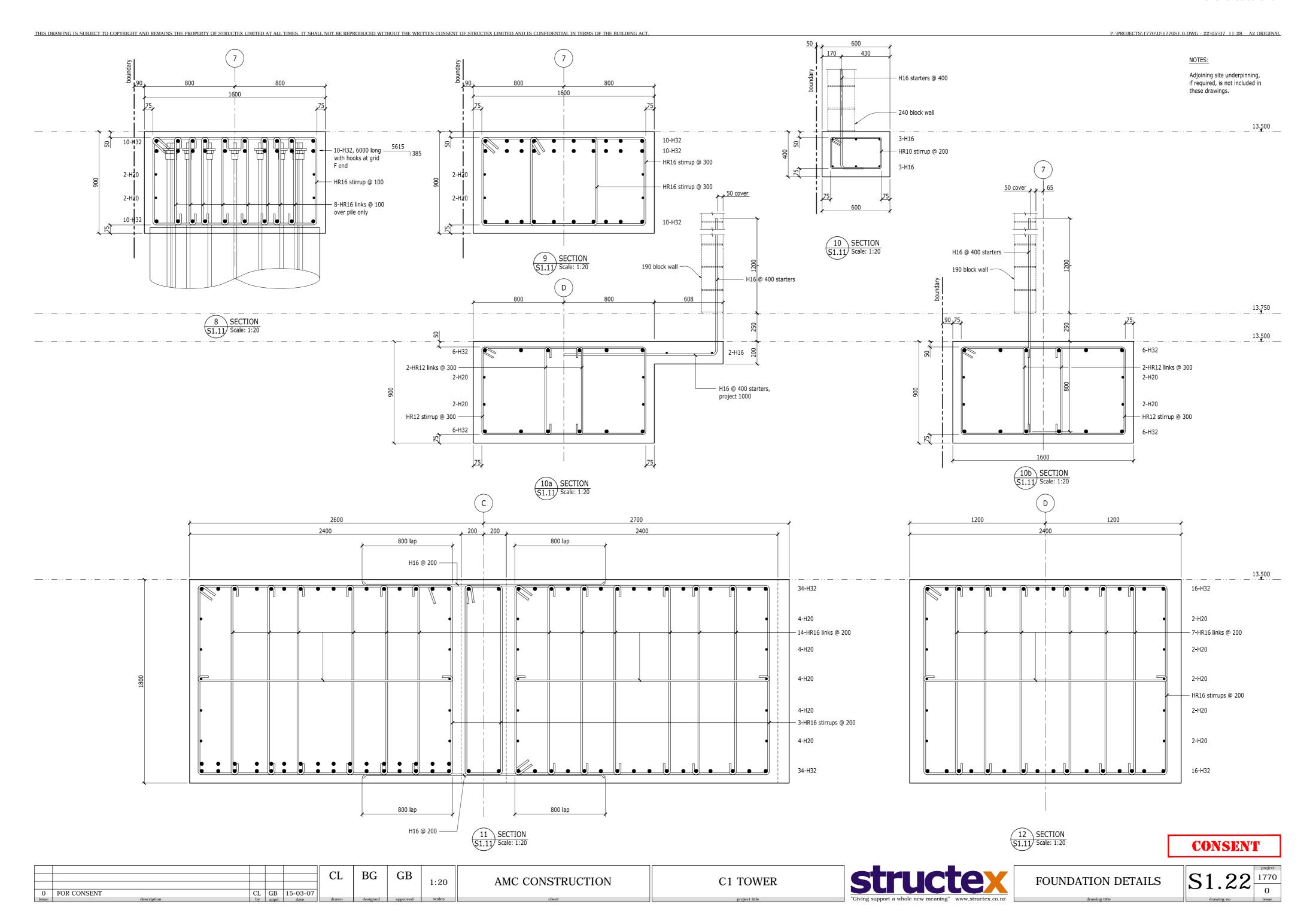


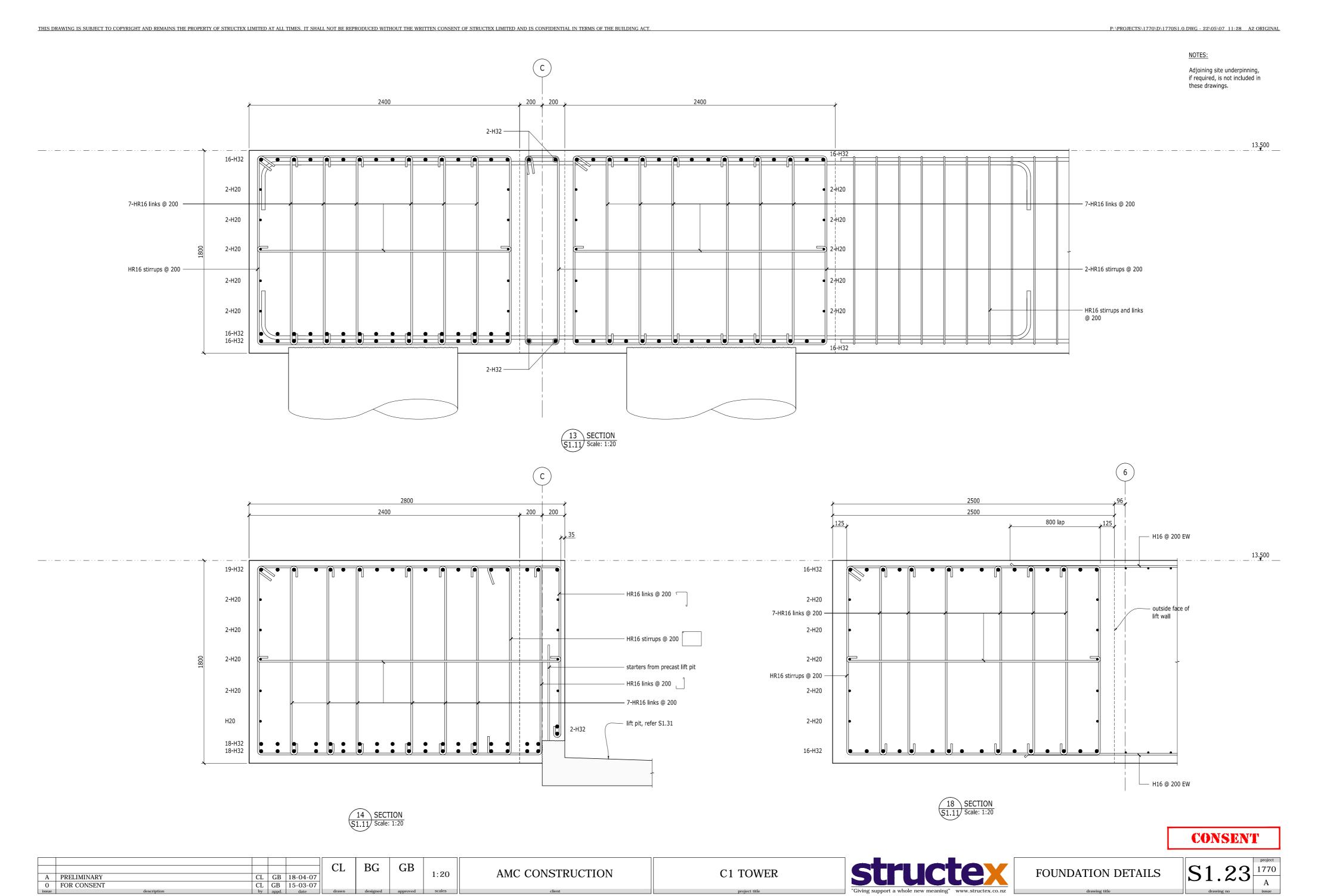
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	DDFF W GOVEDV	CI	an.	10.04.07				1:50	AMC CONSTRUCTION
A	PRELIMINARY	CL	GB	18-04-07					
issue	description	by	appd.	date	drawn	designed	approved	scales	client

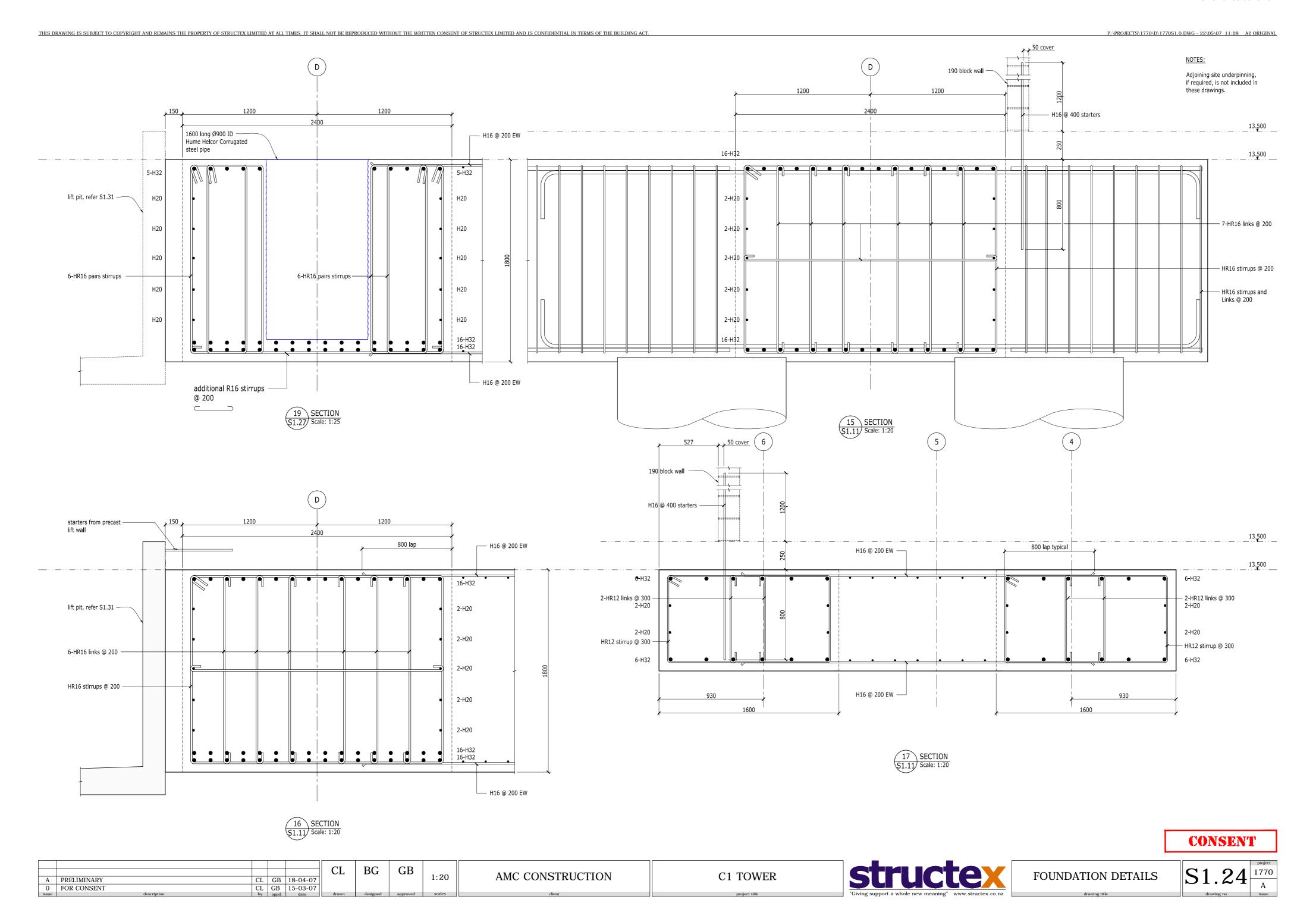


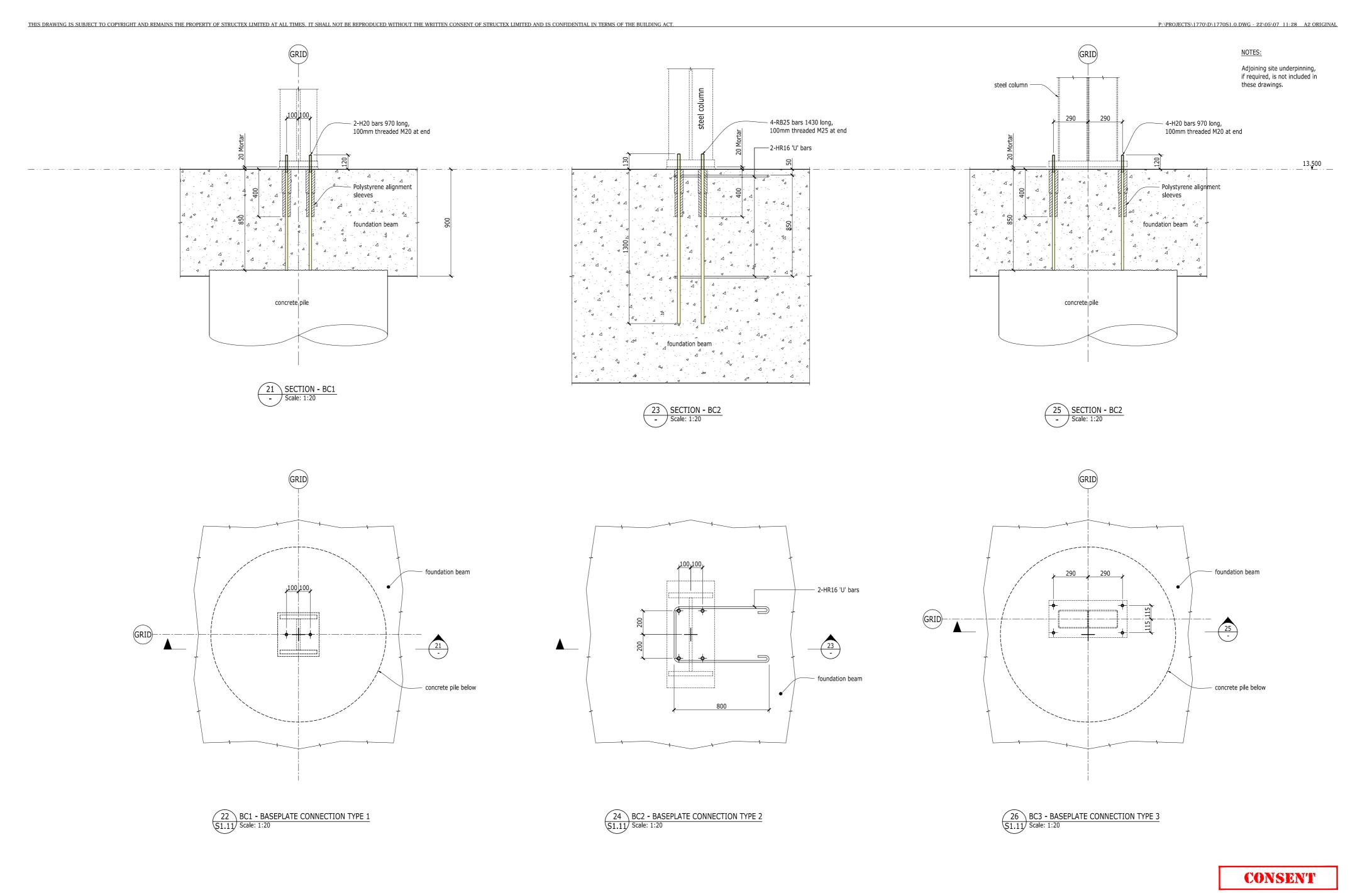
C1 TOWER







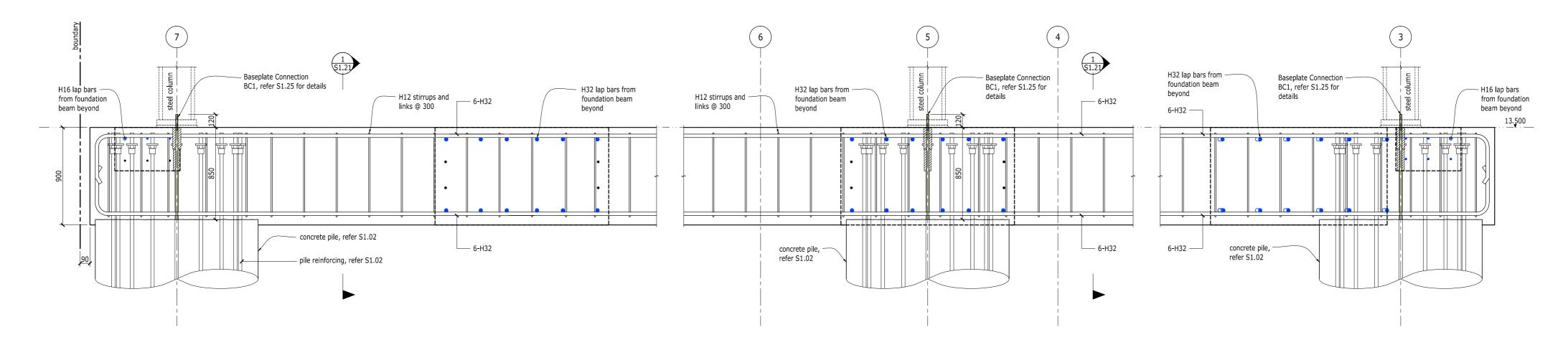




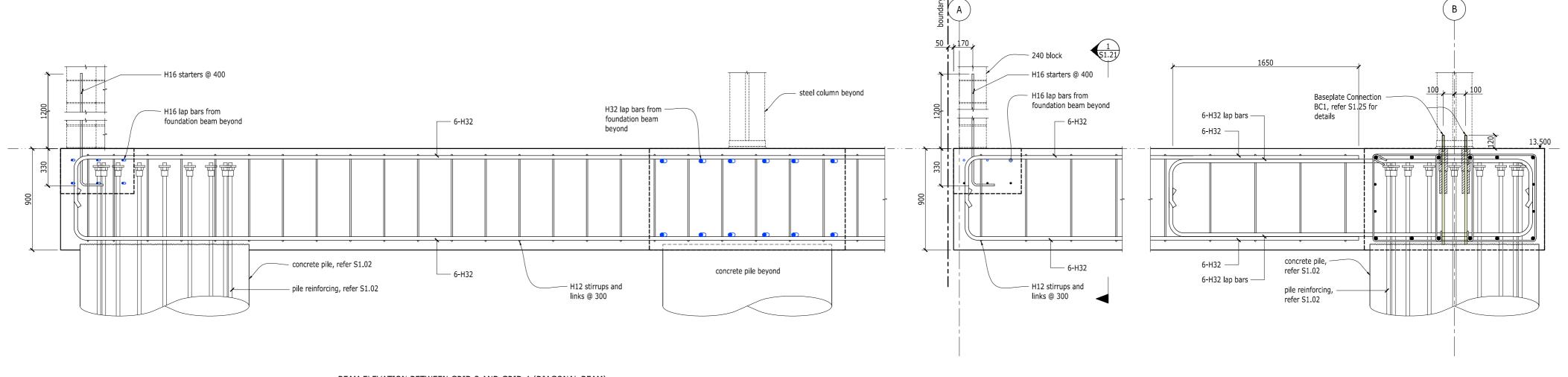
	PRELIMINARY	CL	GB	18-04-07	CL	BG	GB	1:20	AMC CONSTRUCTION	C1 TOWER	structe
)	FOR CONSENT	CI	-	29-03-07							Ju actu
ıe	description	by	appd.	date	drawn	designed	approved	scales	client	project title	"Giving support a whole new meaning" www.structex.



FOUNDATION DETAILS



BEAM ELEVATION GRID B Scale: 1:25



BEAM ELEVATION BETWEEN GRID 3 AND GRID 4 (DIAGONAL BEAM)

Scale: 1:25

BEAM ELEVATION GRID 5 Scale: 1:25

CONSENT

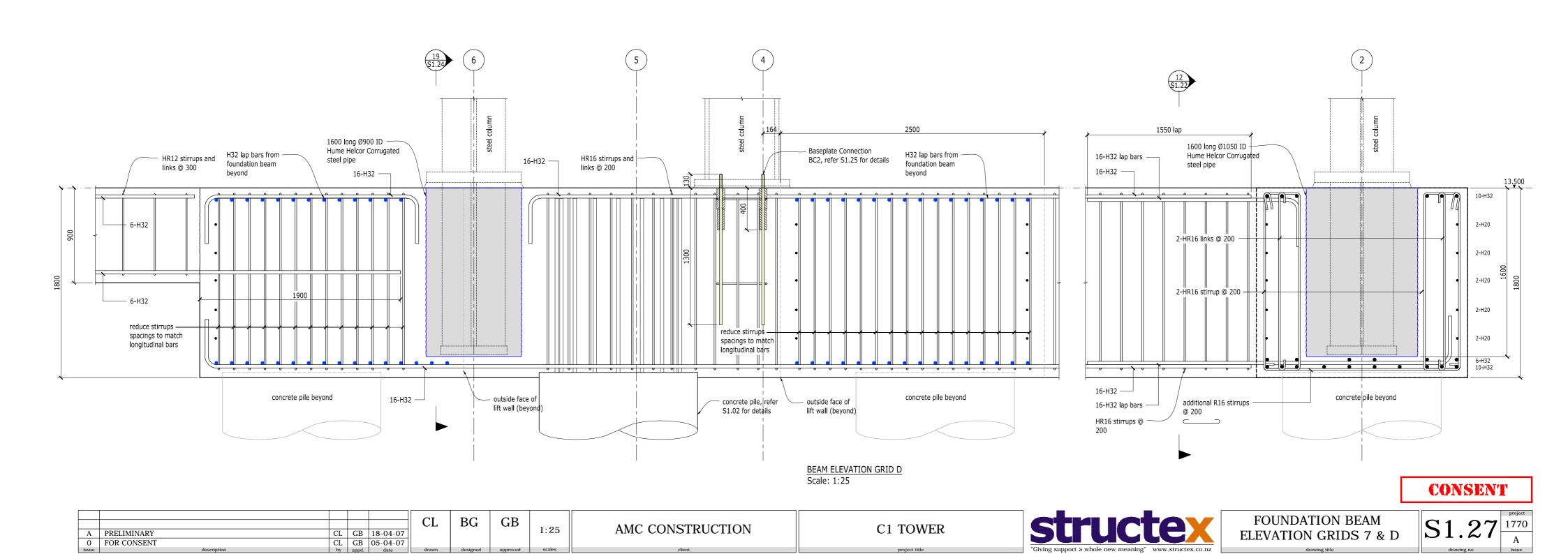
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issue	description	by	appd.	date	drawn	designed	approved	scales	

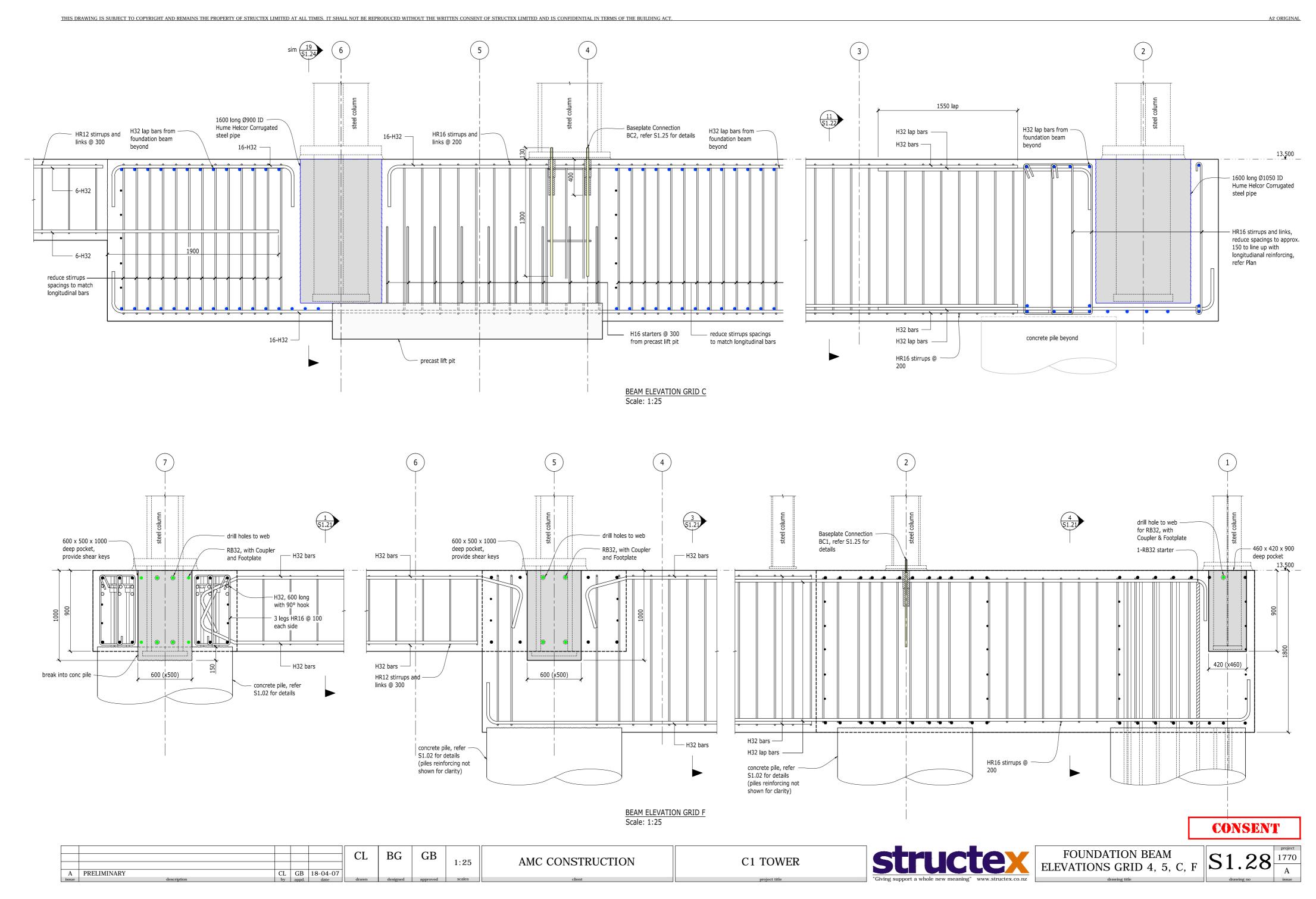


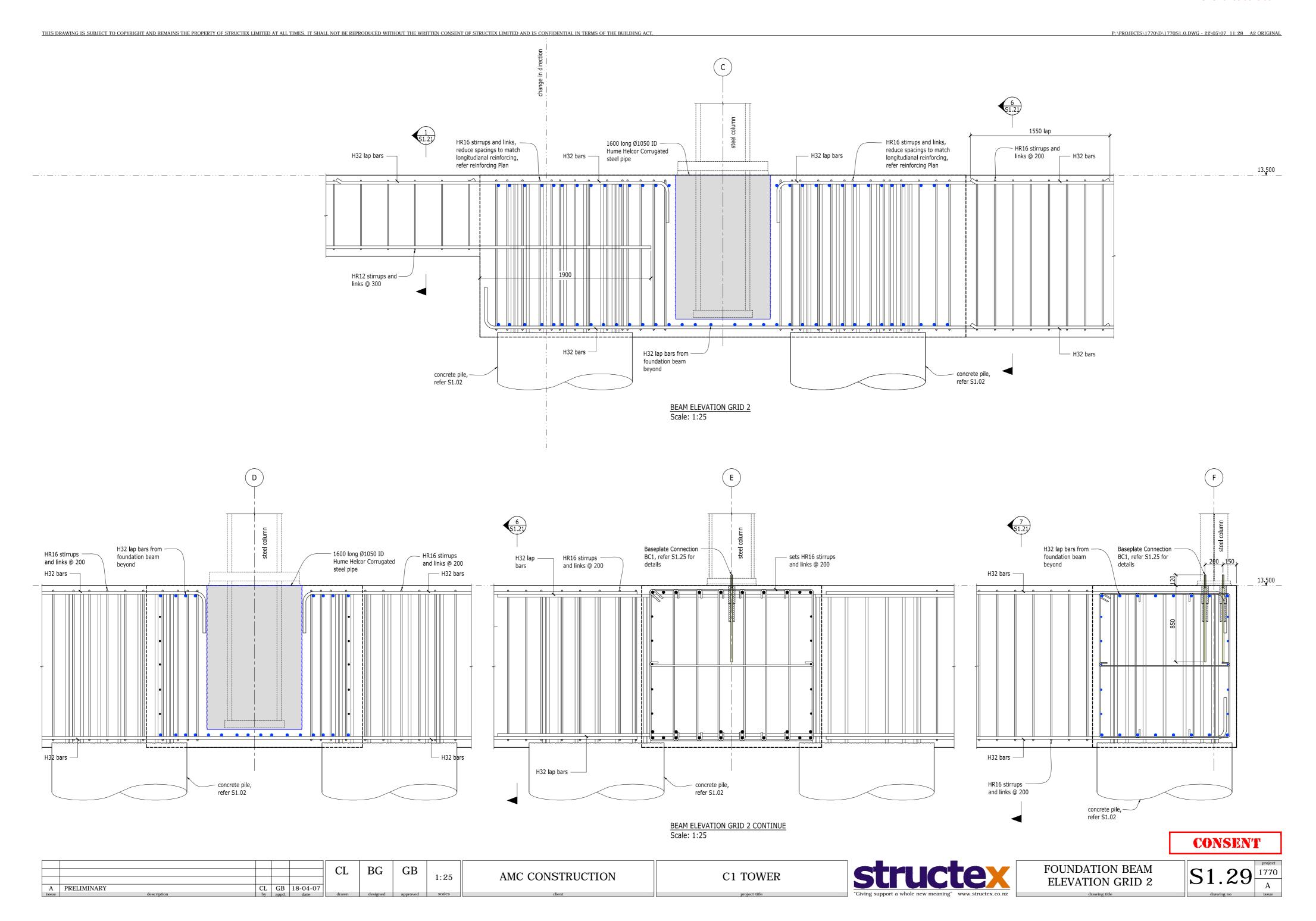


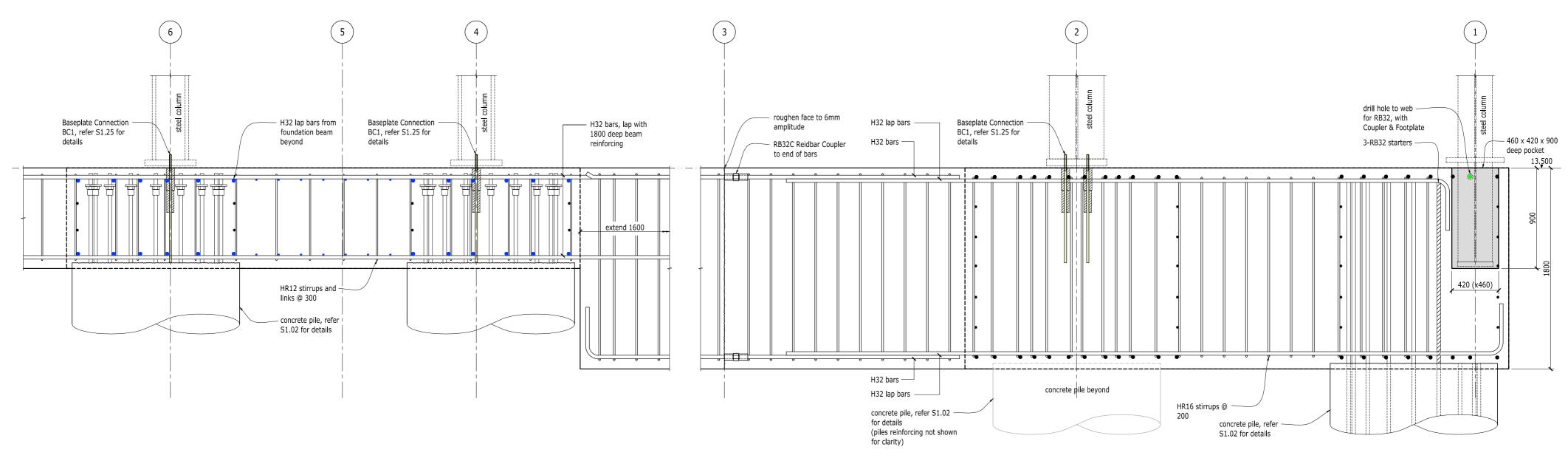


BEAM ELEVATION GRID 7 Scale: 1:25

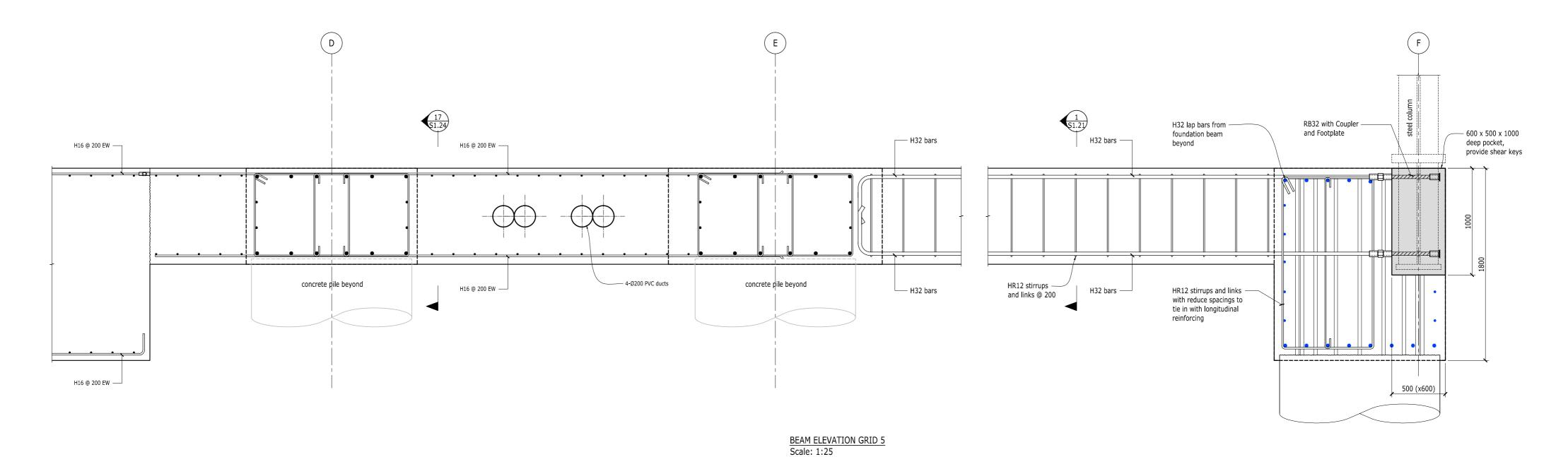








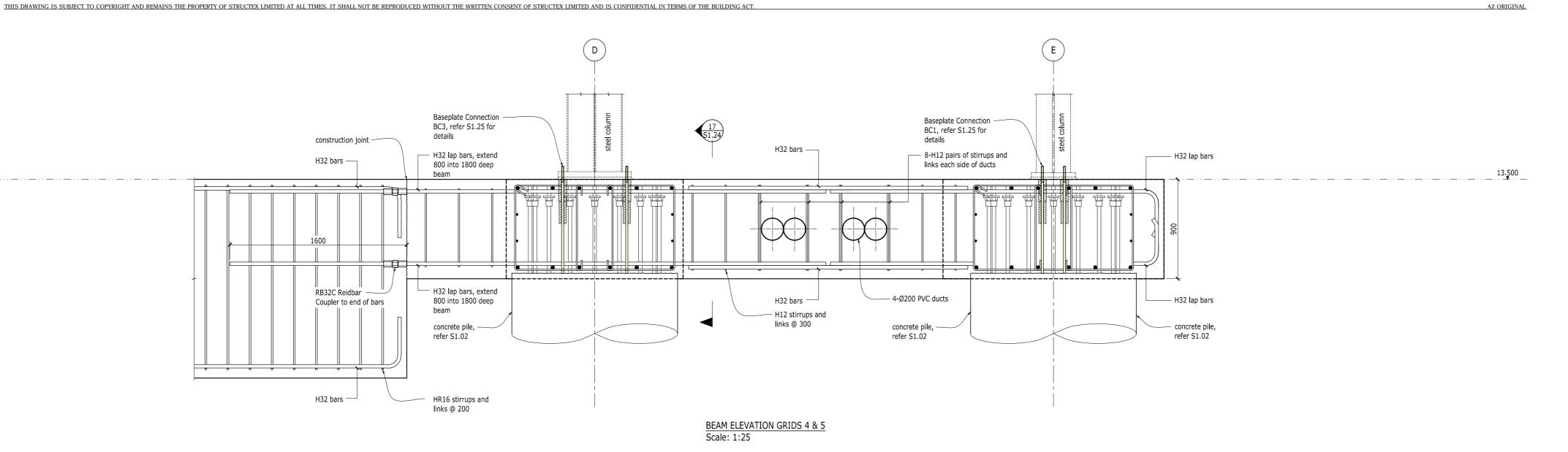
# BEAM ELEVATION GRID E Scale: 1:25



structex

FOUNDATION BEAM ELEVATIONS GRIDS E & 5 S1.30 1770 A issue

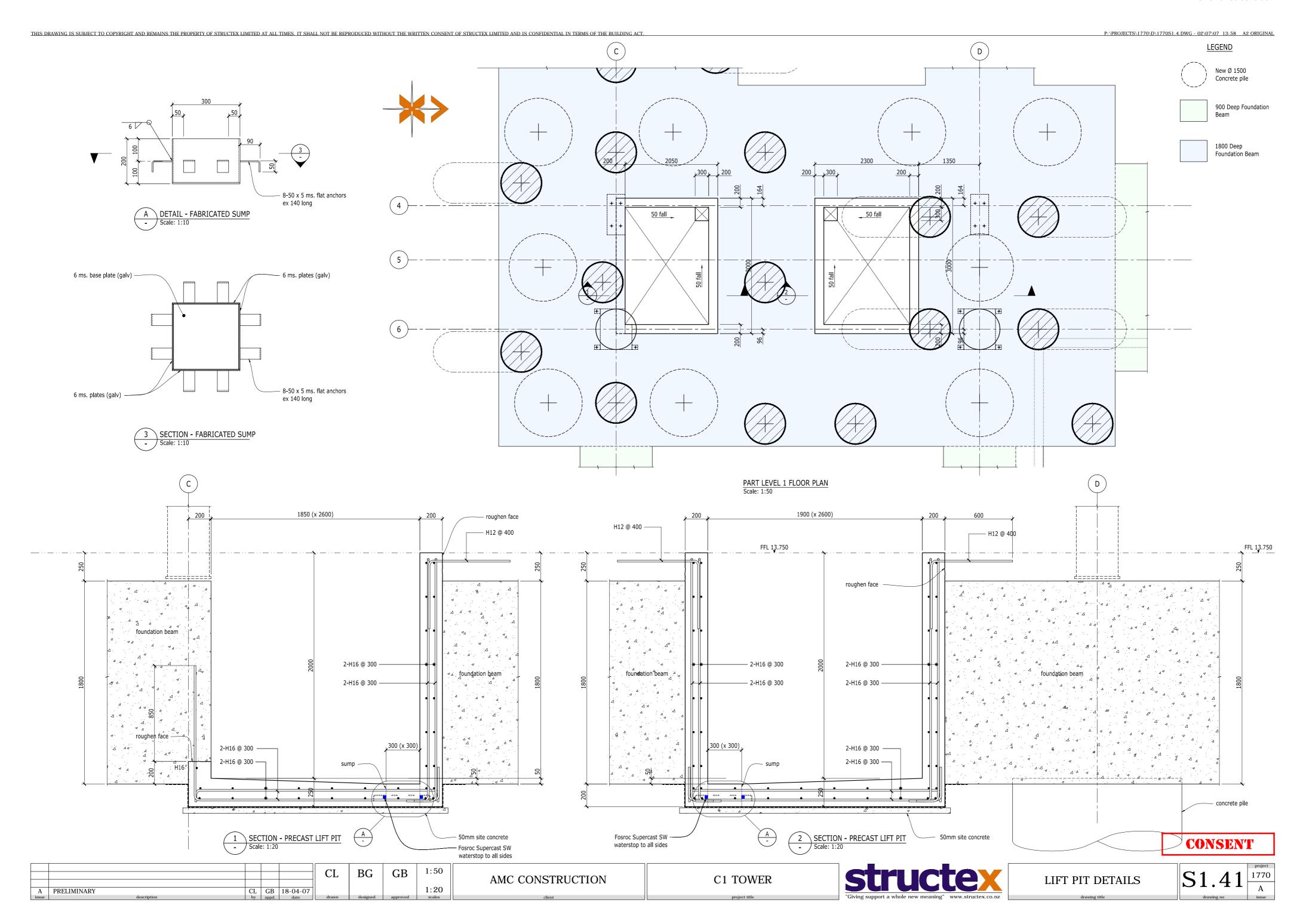
CONSENT

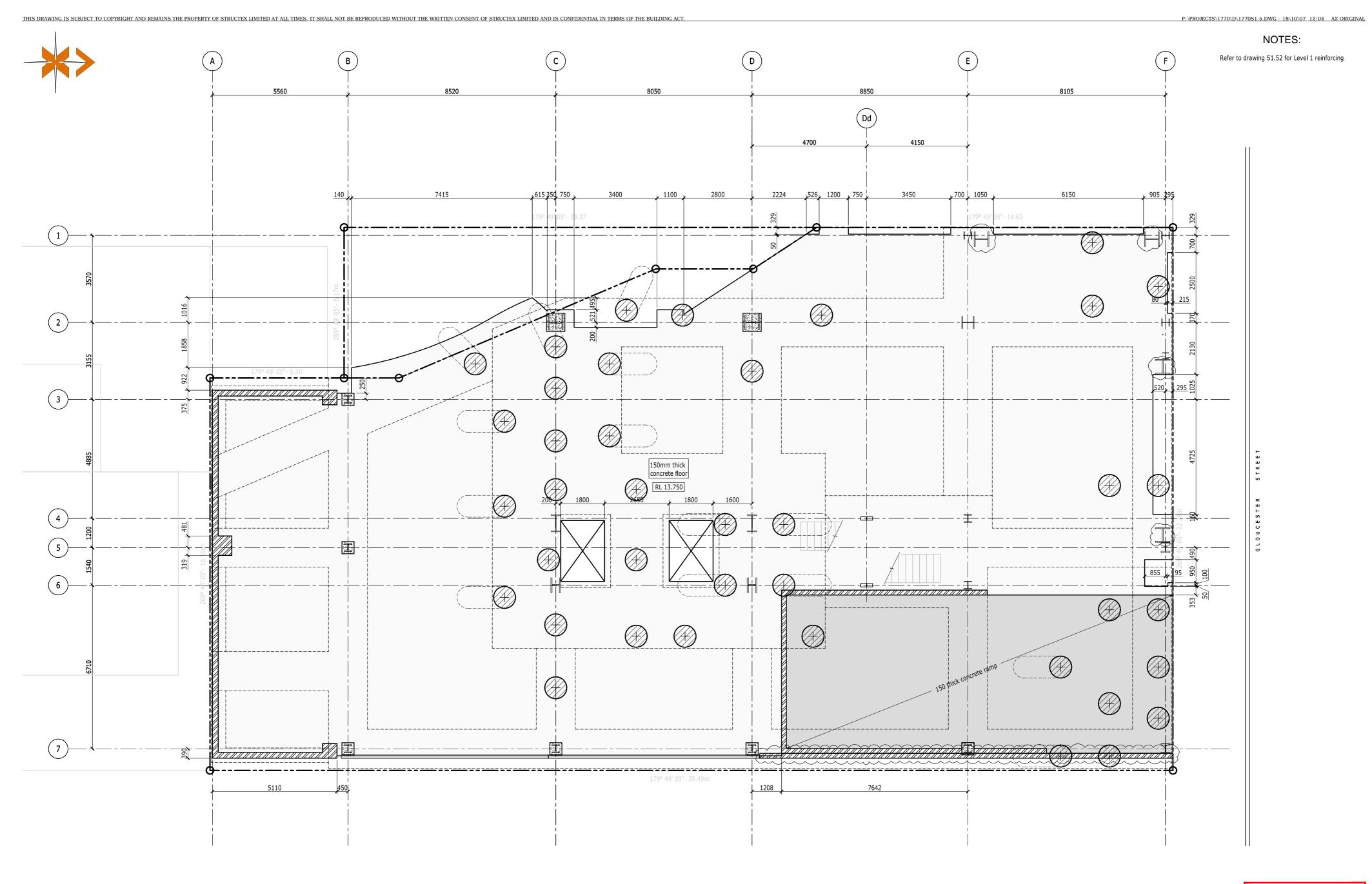


CONSENT









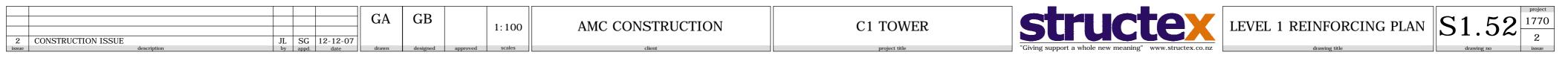
CONSTRUCTION

LEVEL 1 PLAN

S1.51

	GA	GB	1:100	AMC CONSTRUCTION	C1 TOWER	structex	
CONSTRUCTION ISSUE   JL   SG   12-12-07							
description by appd. date	drawn	designed approved	scales	client	project title	Giving support a whole new meaning www.structex.co.nz	

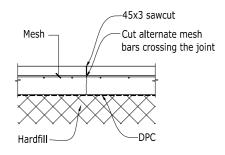
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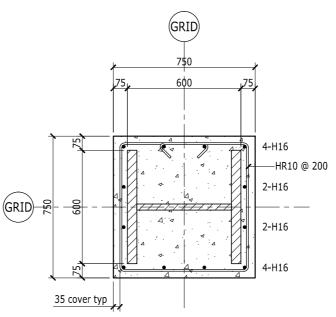
P:\PROJECTS\1770\D\1770S1.5.DWG - 18\10\07 12:04 A2 ORIGINAL

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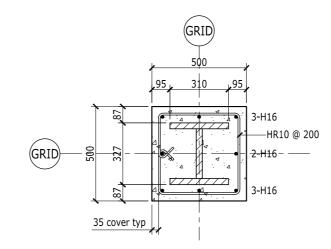




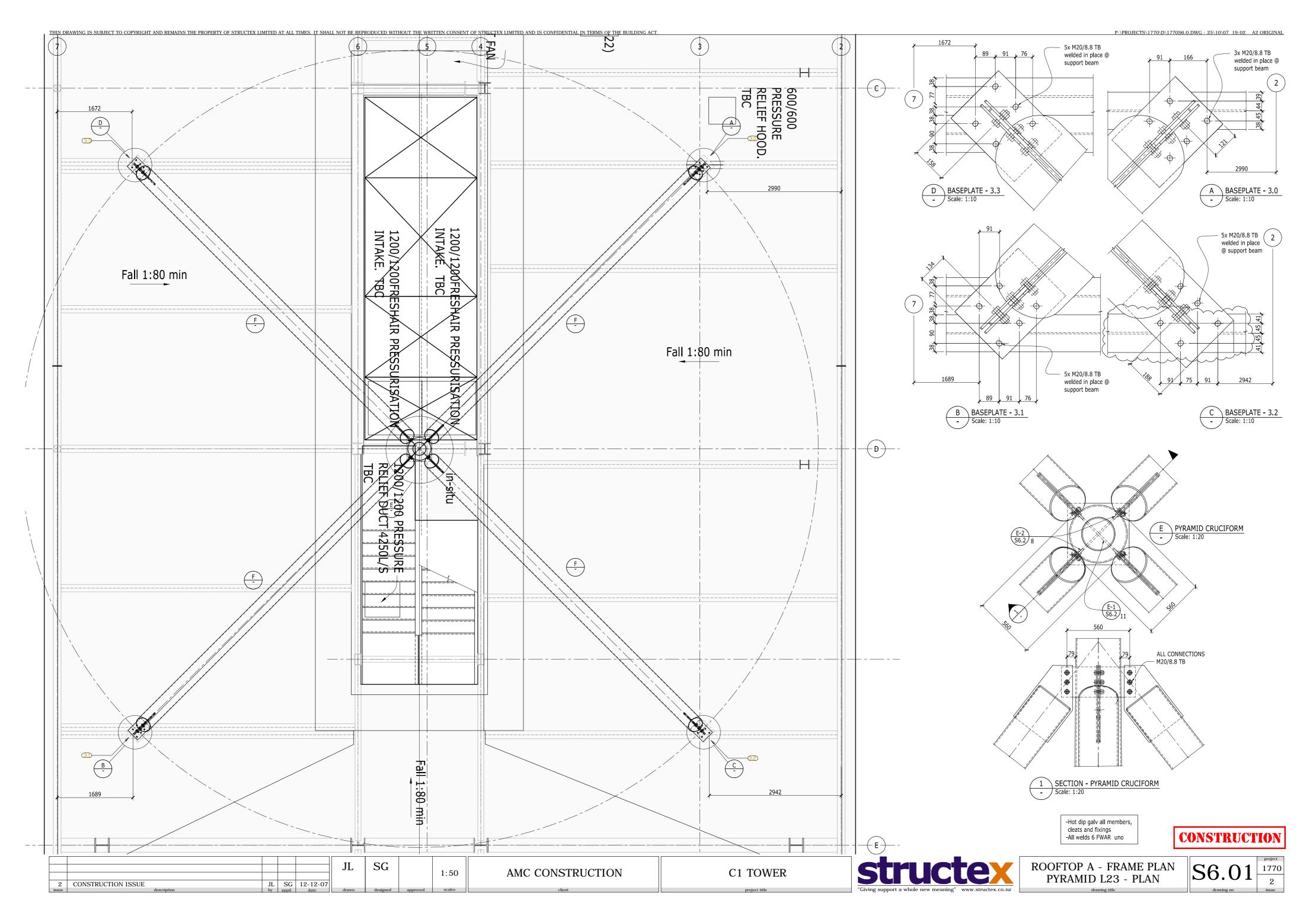
(CJ1) UNFILLED SAWCUT JOIN

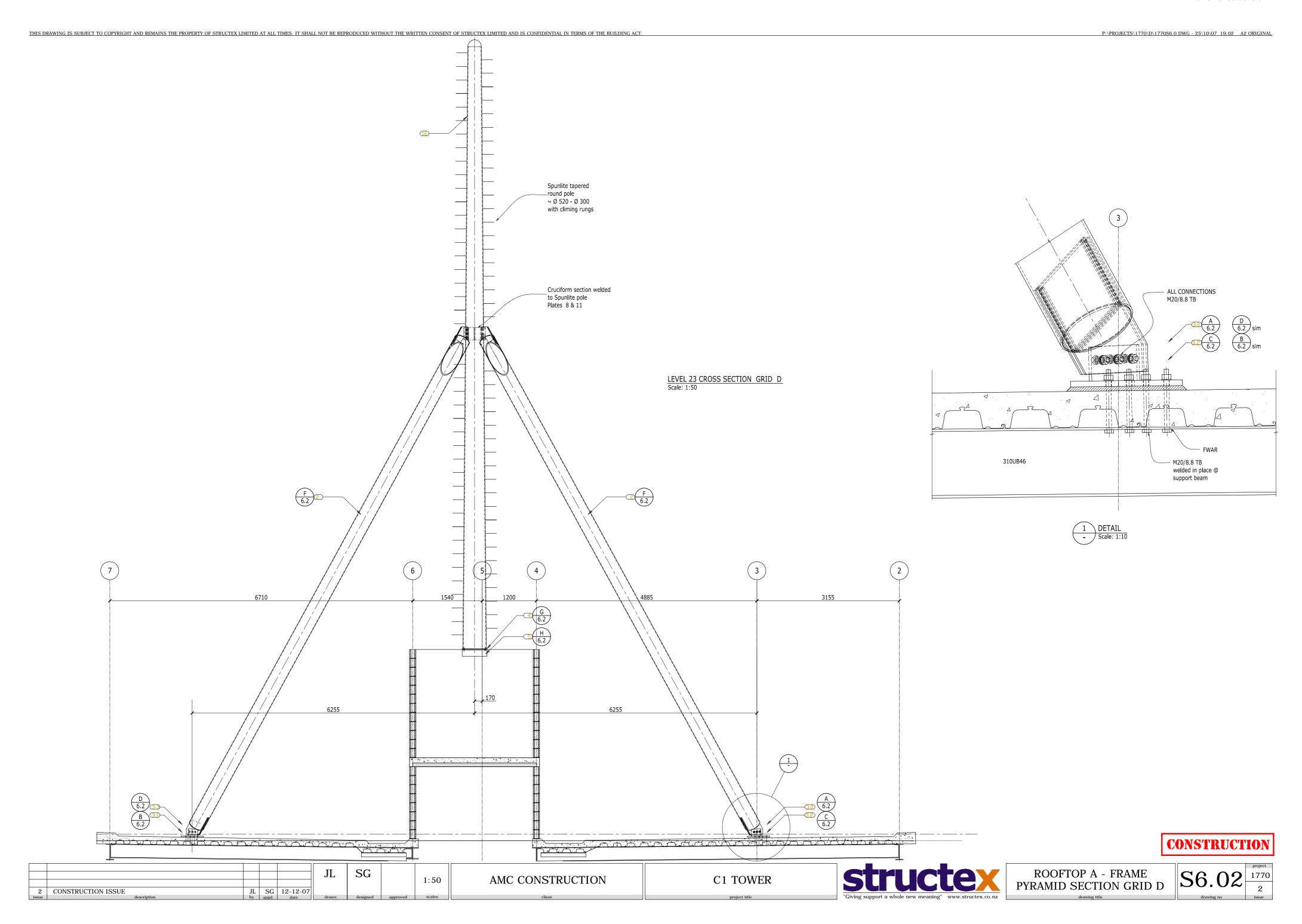


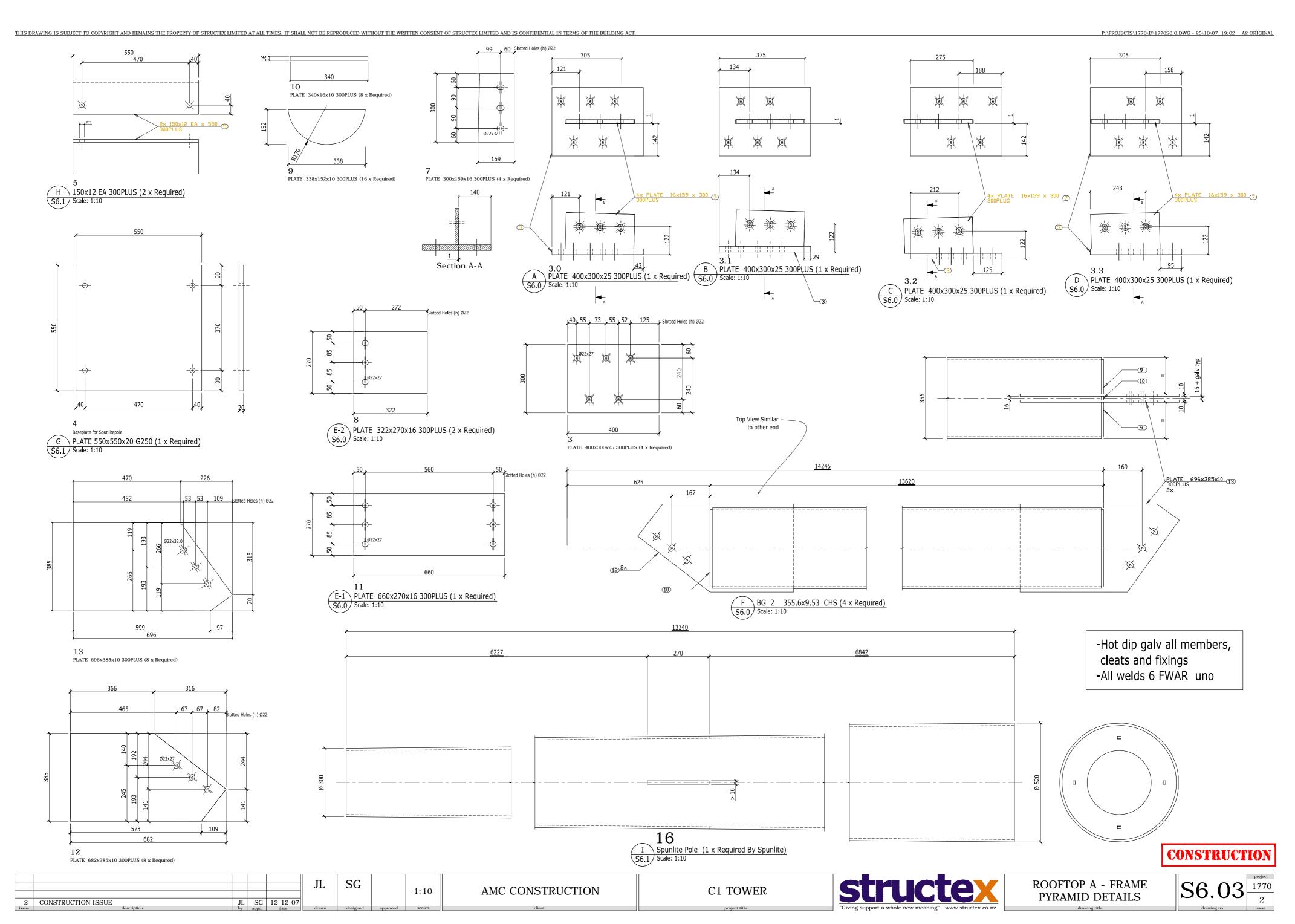
COLUMN ENCASEMENT - GRIDS C2, D2 - LEVEL 1
Scale: 1:20

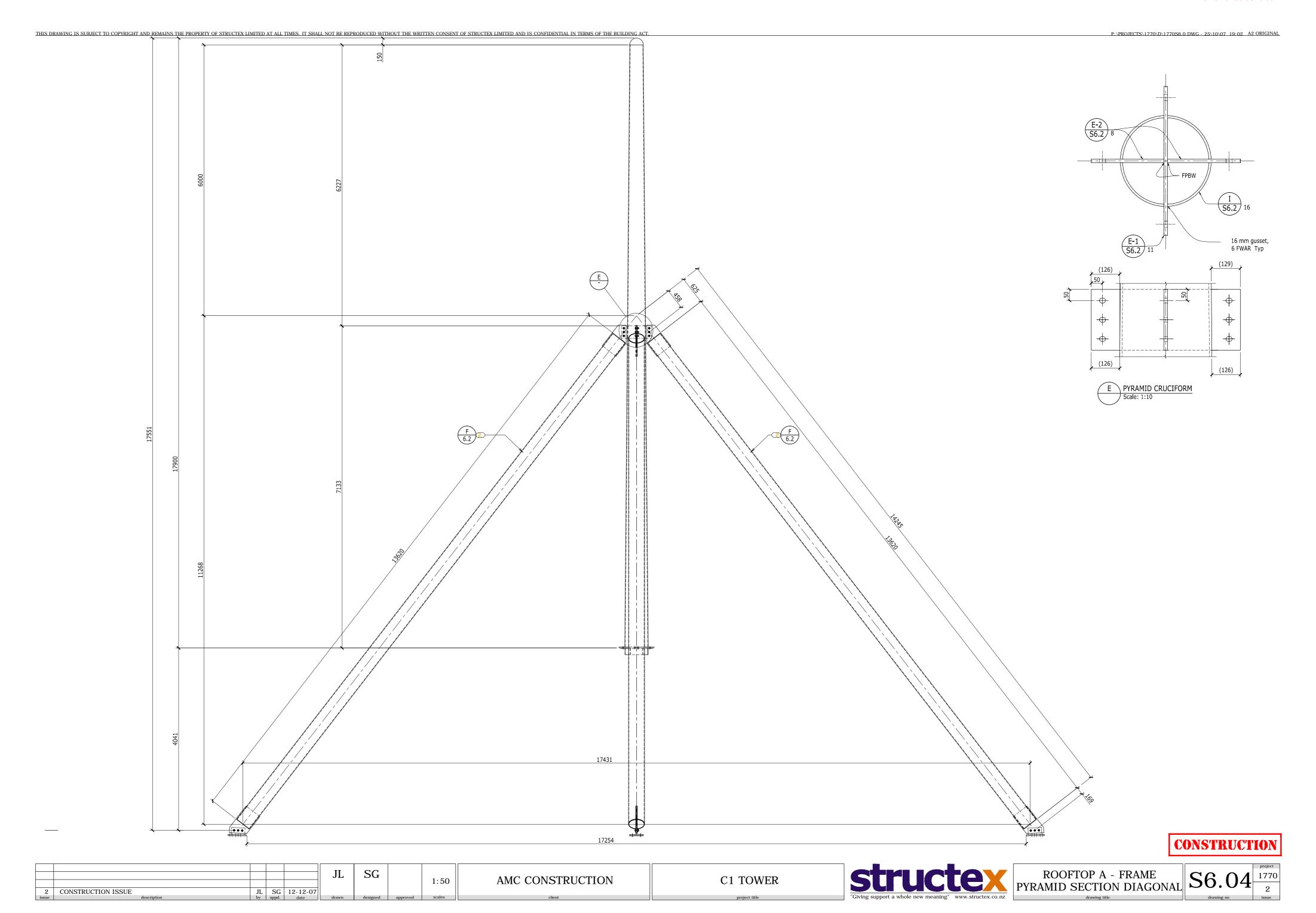


COLUMN ENCASEMENT - GRIDS B3, B5, B7, C7, D7, E7 - LEVELS 1-6 Scale: 1:20



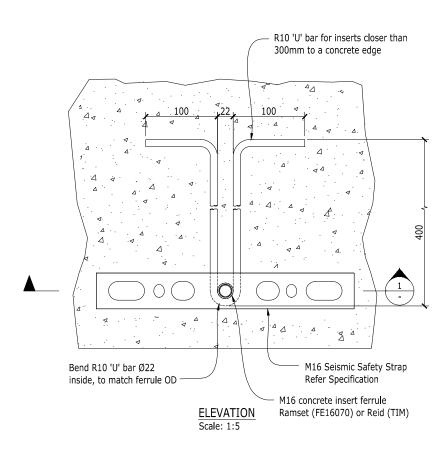


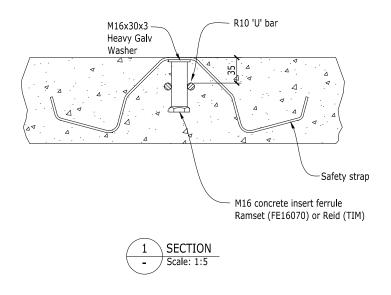




Minimum cover to reinforcement to be 30mm

For typical stair reinforcement refer: Lower Stair - S2.75 Upper Stair - S2.73

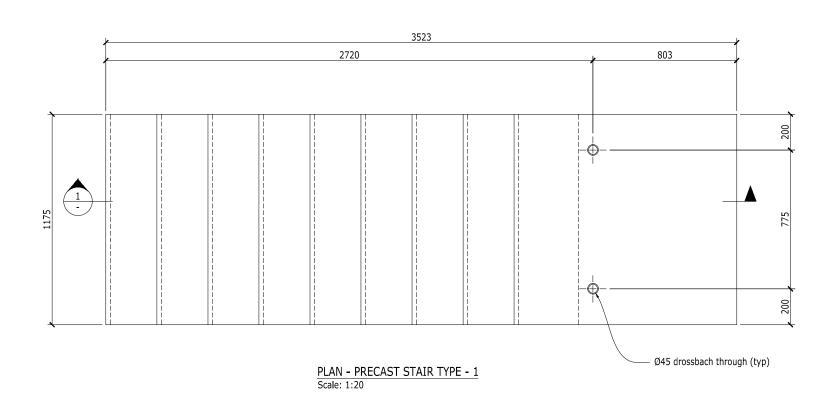


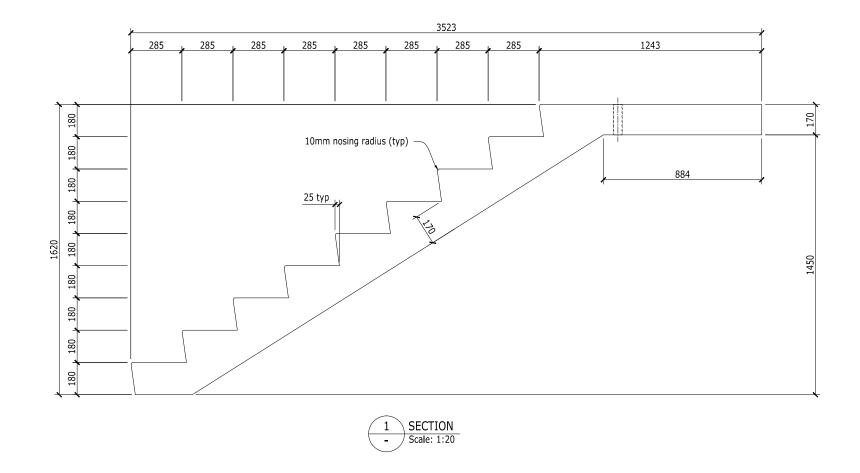


SEISMIC SAFETY STRAP - ARRANGEMENT Scale: 1:5

TOWER STRUCTEX

Refer to drawing S2.70 for notes and details



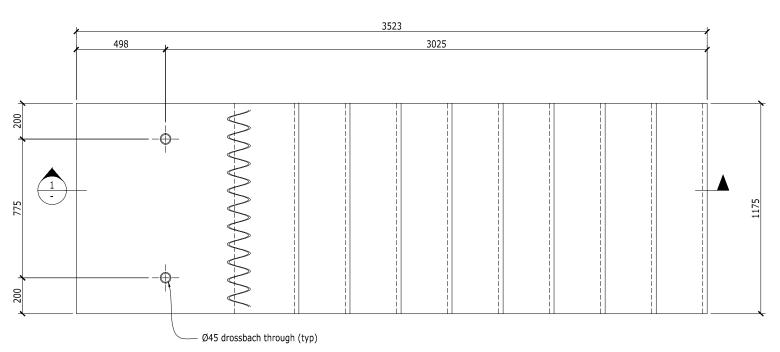




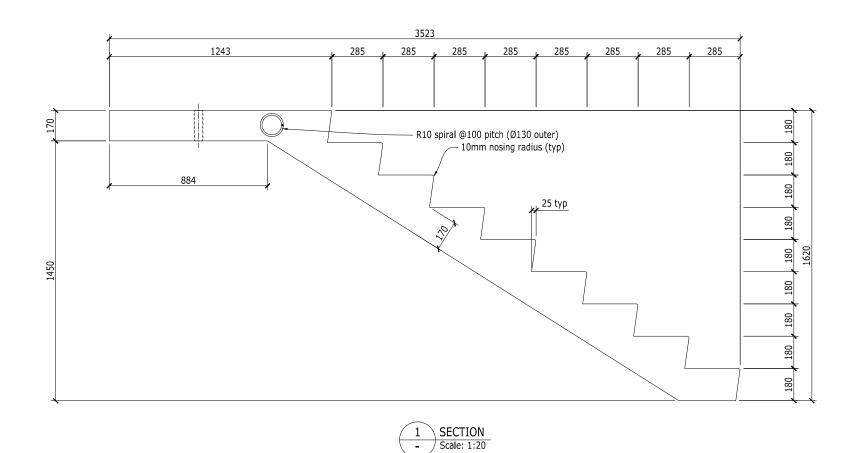




Refer to drawing S2.70 for notes and details



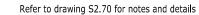
PLAN - PRECAST STAIR TYPE - 2 Scale: 1:20

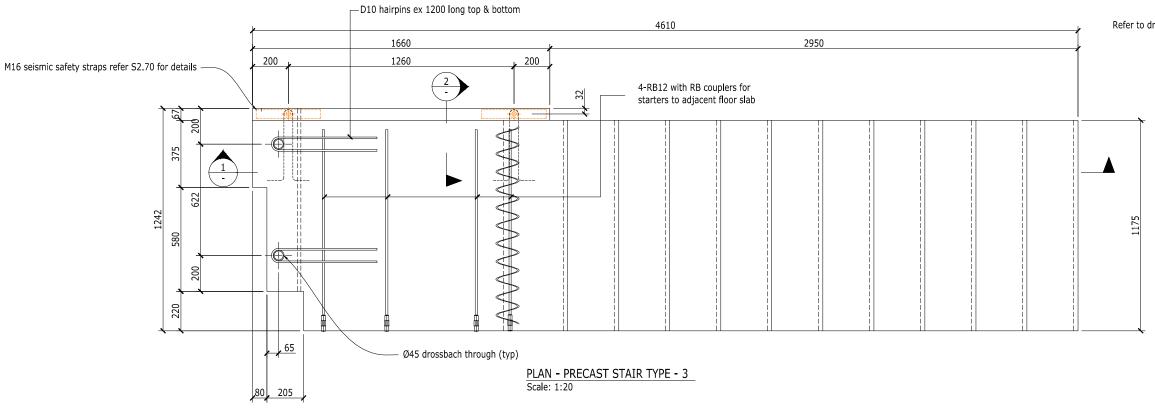


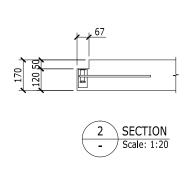


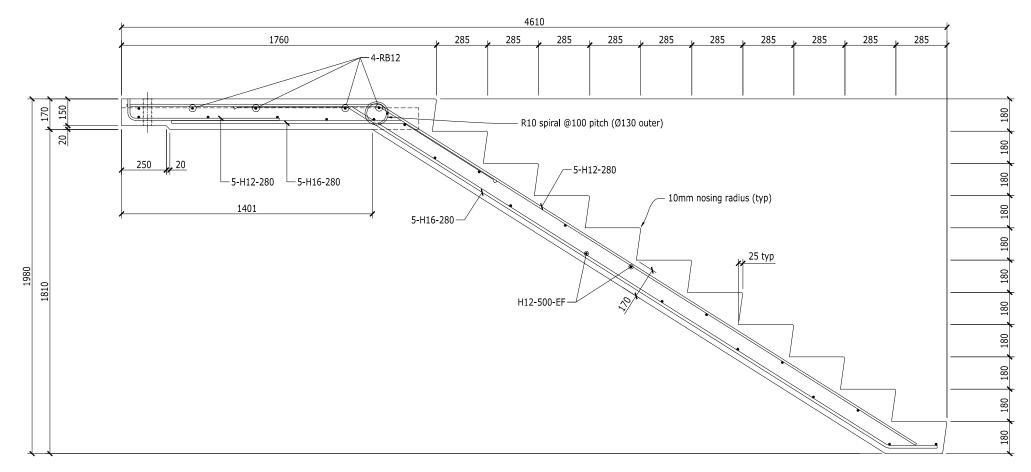














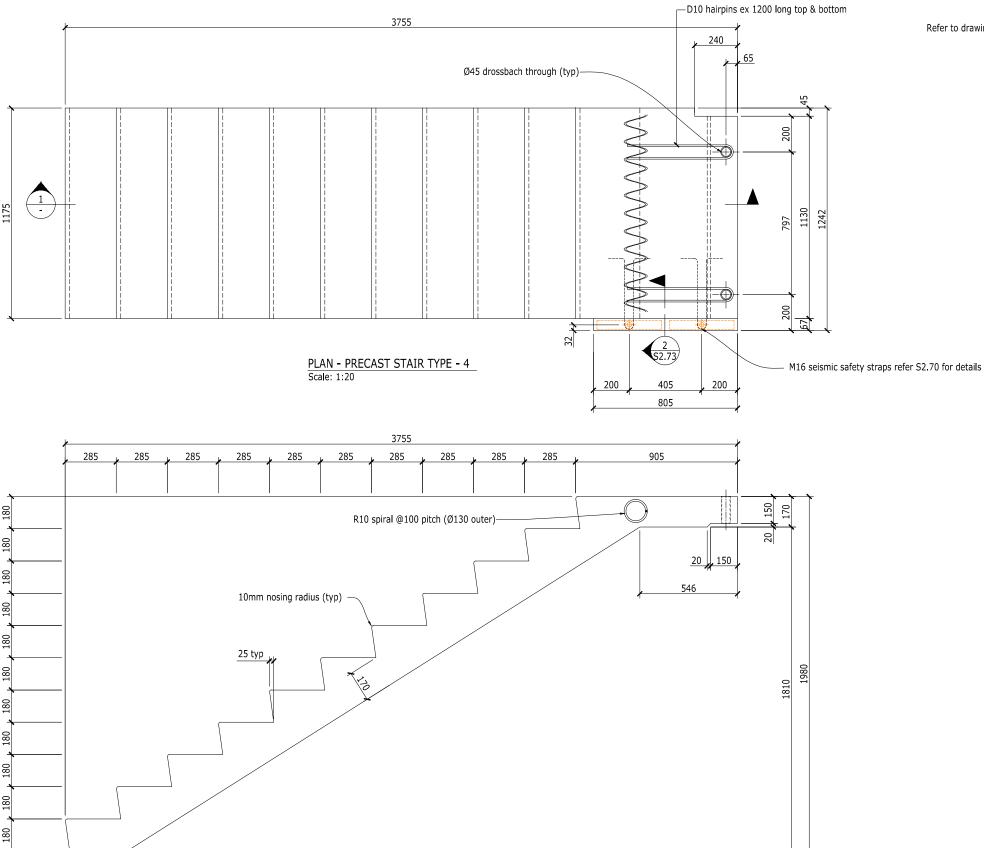
CONSTRUCTION

1:20 GA GB AMC CONSTRUCTION C1 TOWER 2 CONSTRUCTION ISSUE JL SG 12-12-07





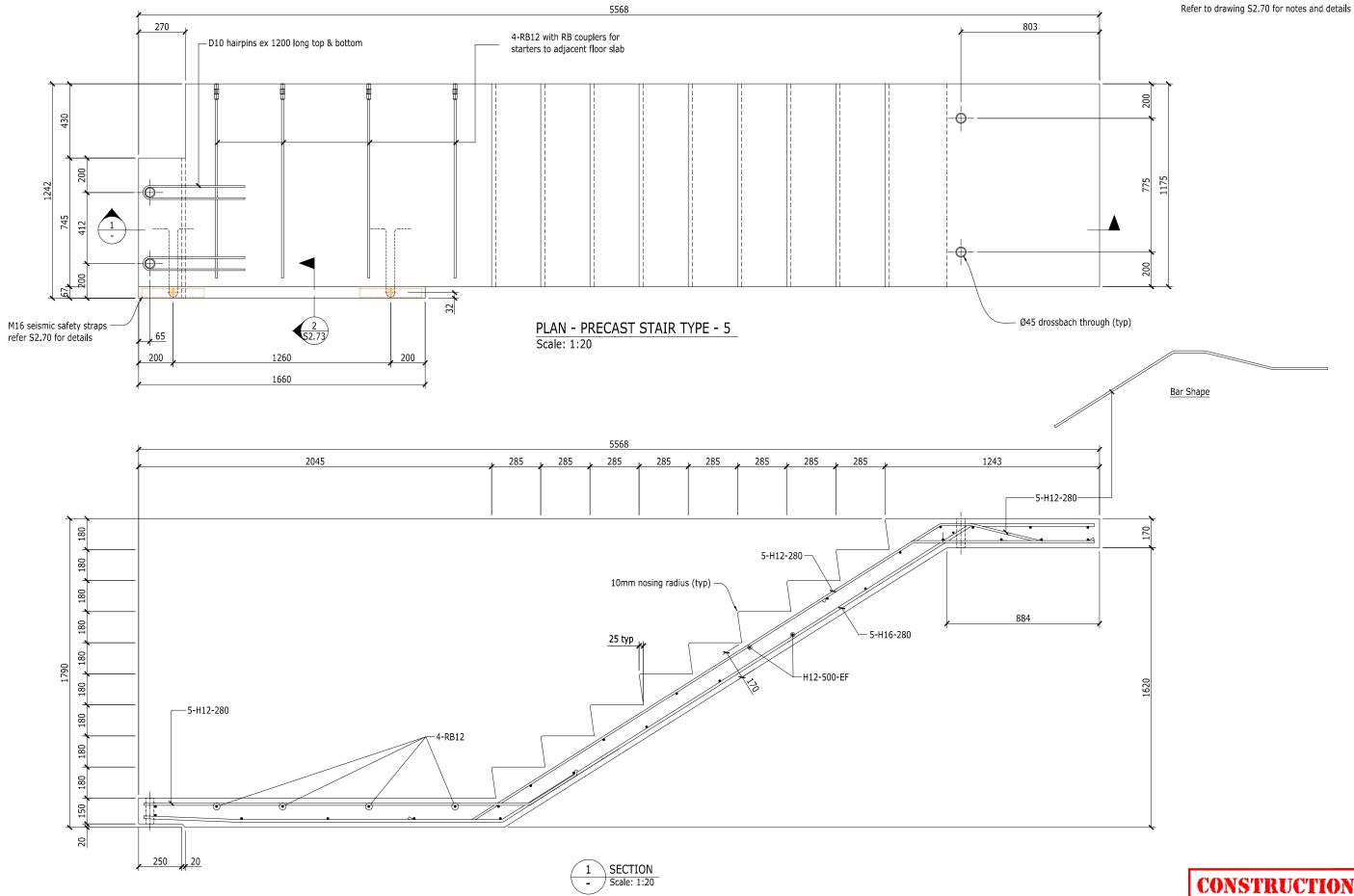
Refer to drawing S2.70 for notes and details









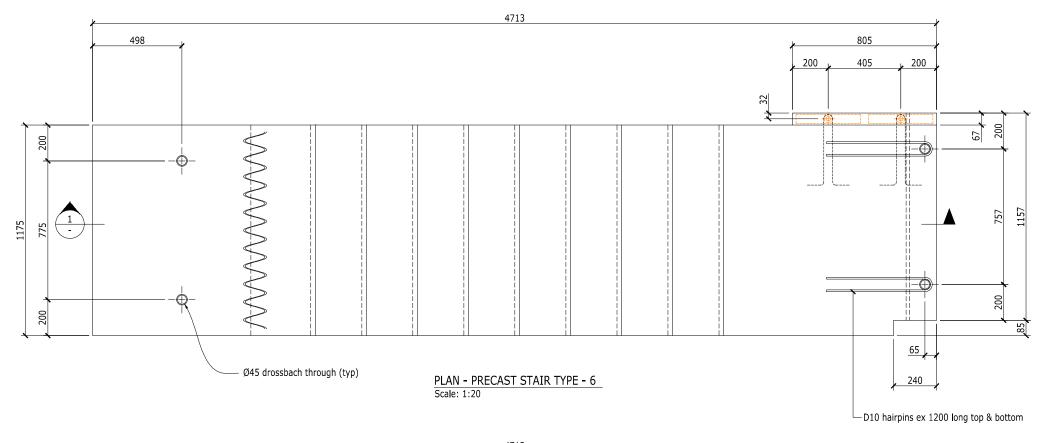


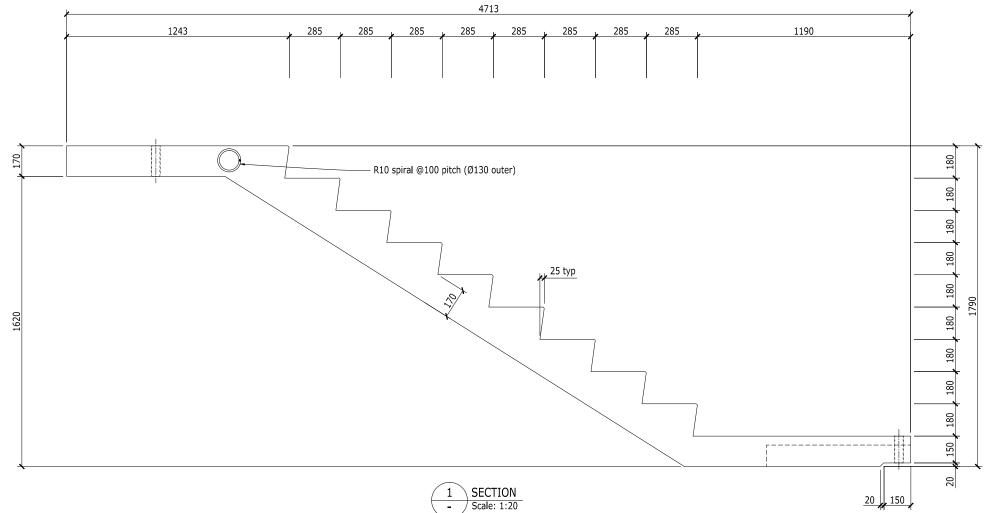
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PRECAST STAIR TYPE 5

Refer to drawing S2.70 for notes and details



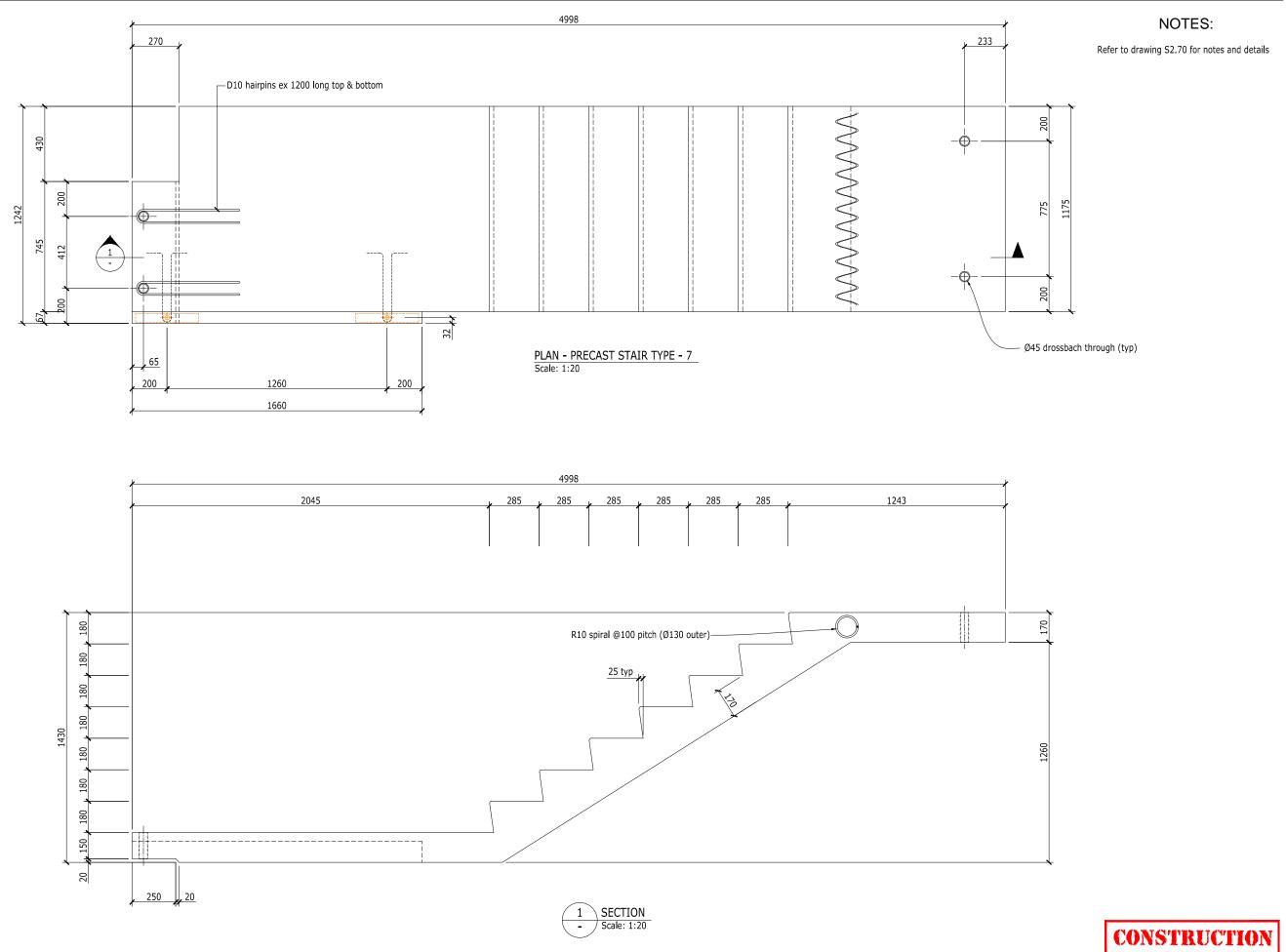


CONSTRUCTION

1:20 GA GB AMC CONSTRUCTION C1 TOWER JL SG 12-12-07 by appd. date 2 CONSTRUCTION ISSUE



2 CONSTRUCTION ISSUE

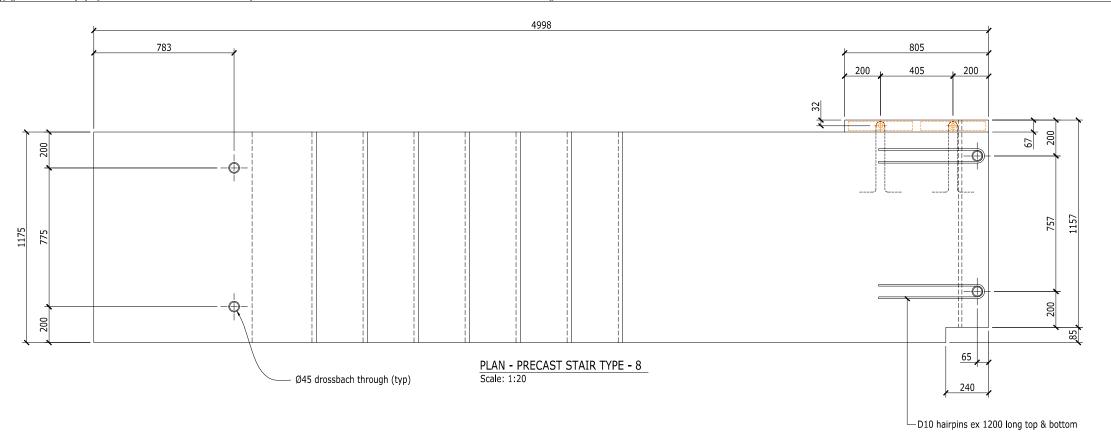


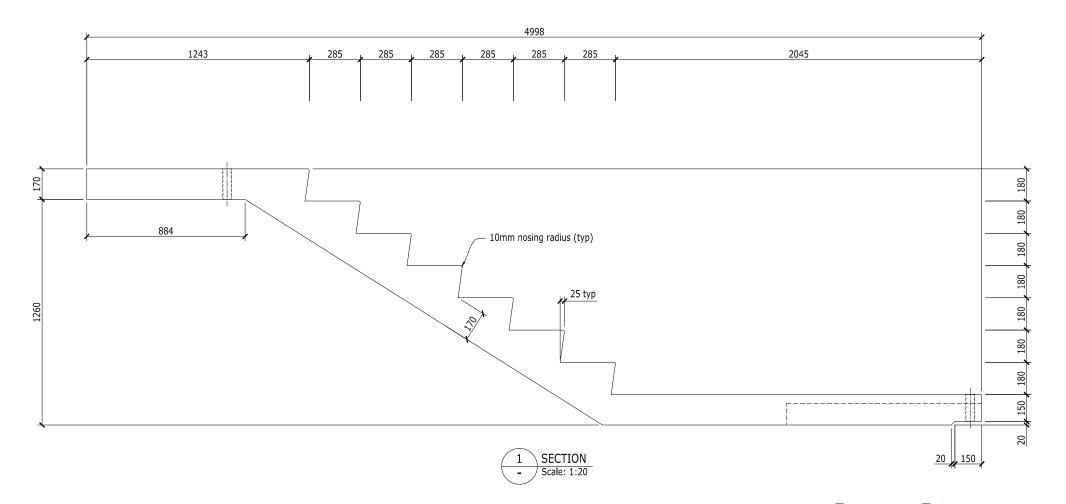
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PRECAST STAIR TYPE 7

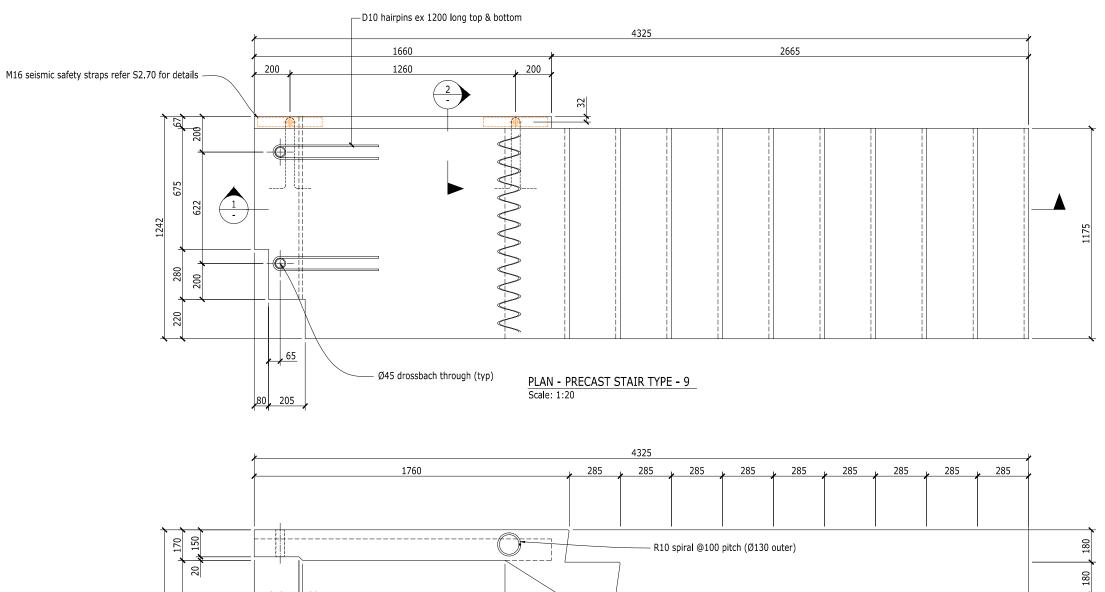
Refer to drawing S2.70 for notes and details

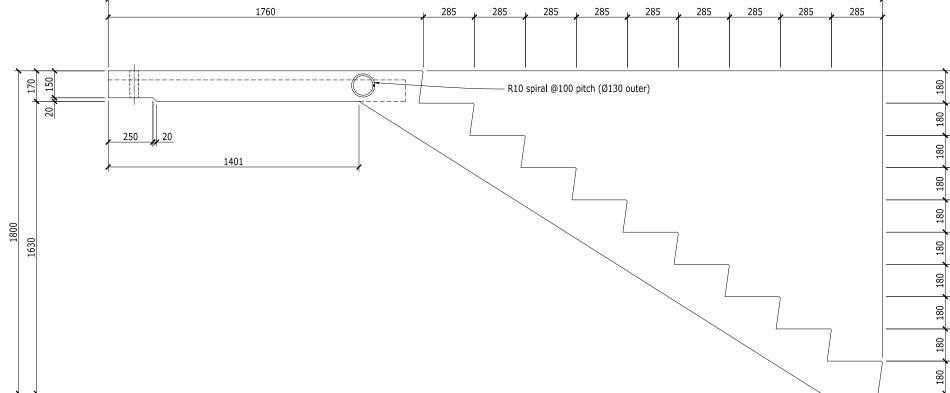






Refer to drawing S2.70 for notes and details



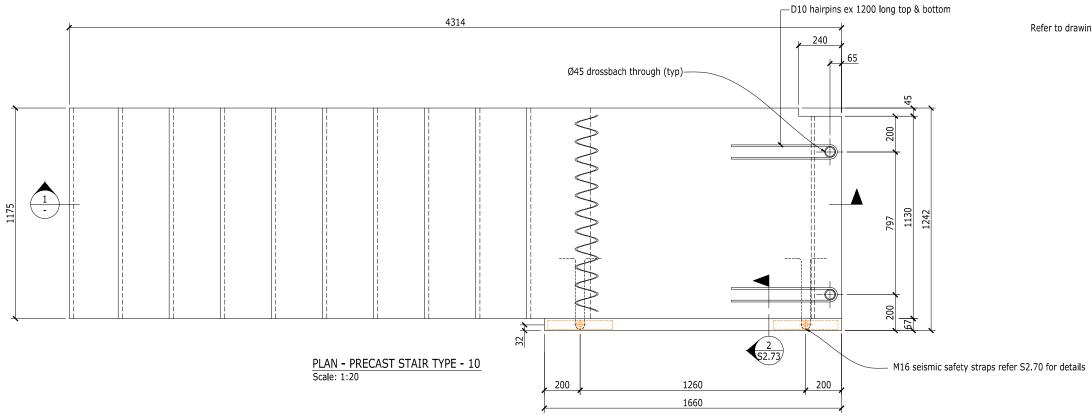


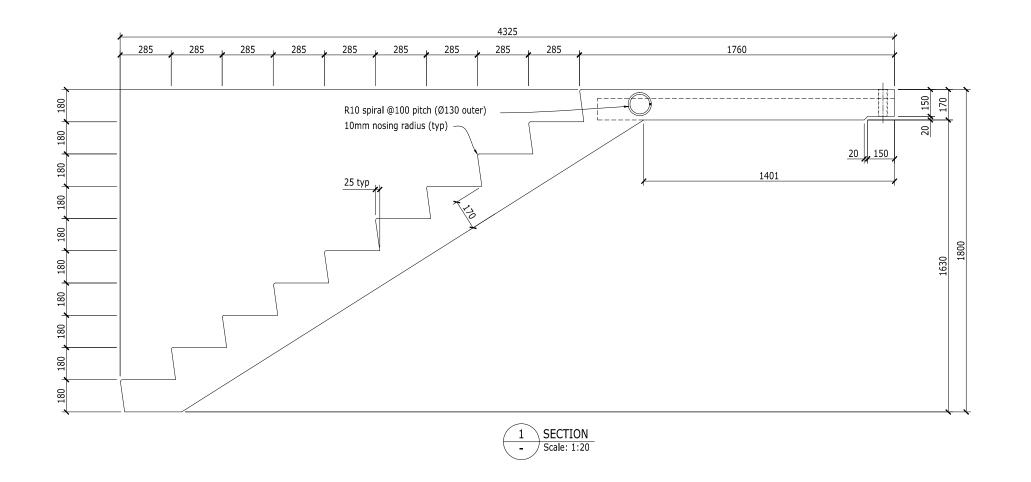






Refer to drawing S2.70 for notes and details



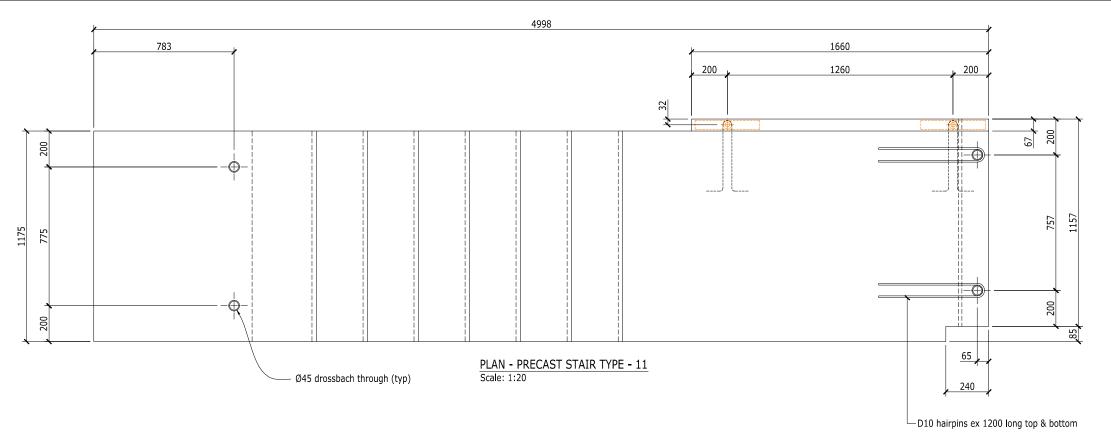


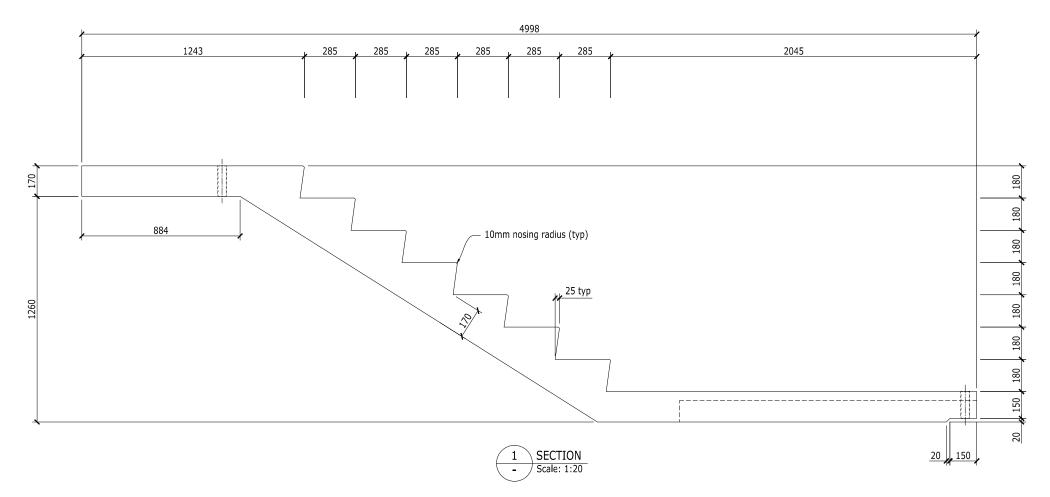






Refer to drawing S2.70 for notes and details





1:20

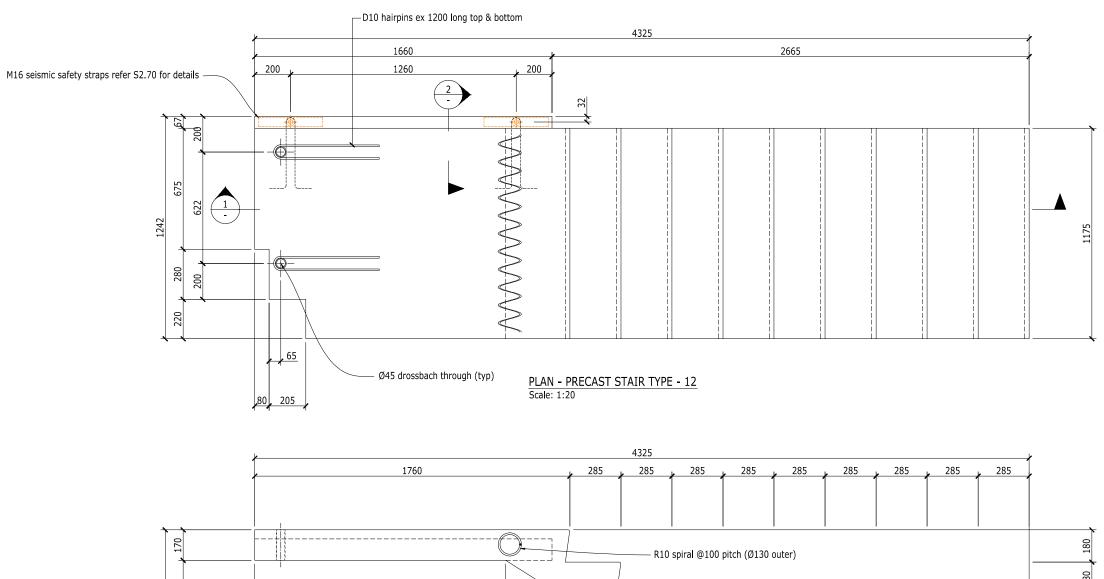
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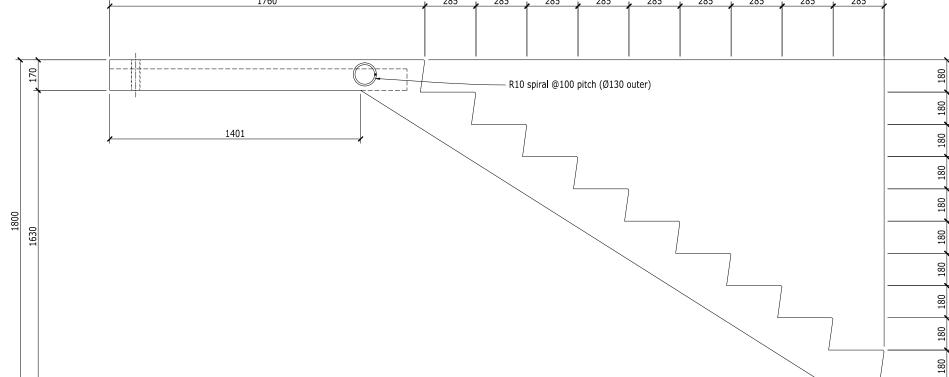
JL SG 12-12-07 by appd. date

2 CONSTRUCTION ISSUE

GB

Refer to drawing S2.70 for notes and details







C1 TOWER

AMC CONSTRUCTION

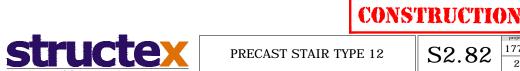
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GA

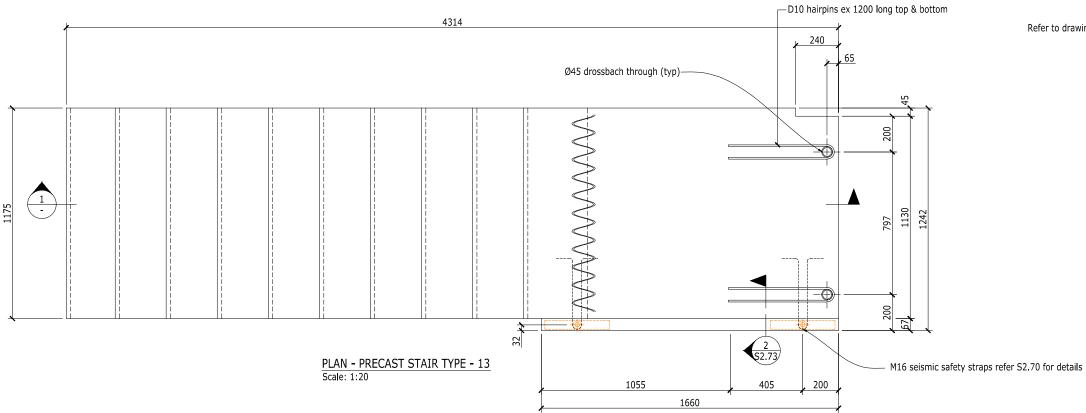
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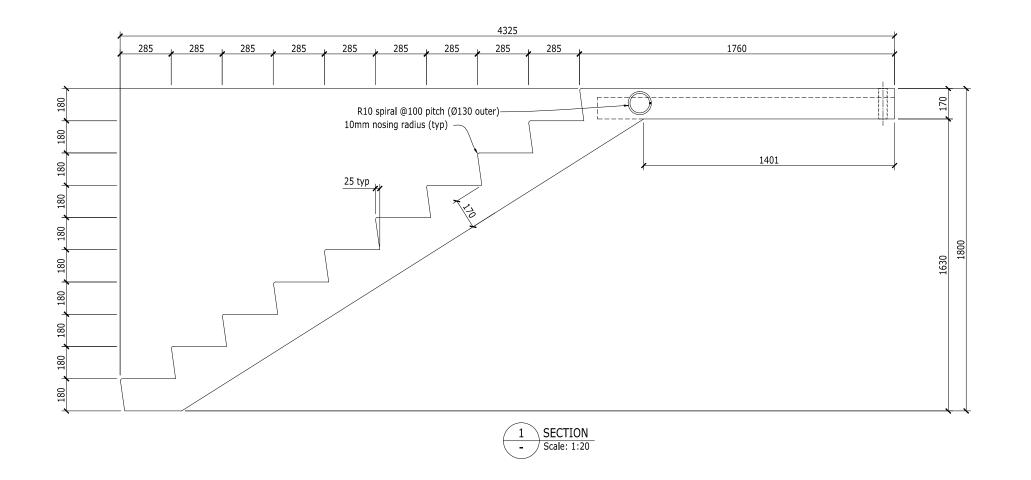
2 CONSTRUCTION ISSUE

GB



Refer to drawing S2.70 for notes and details

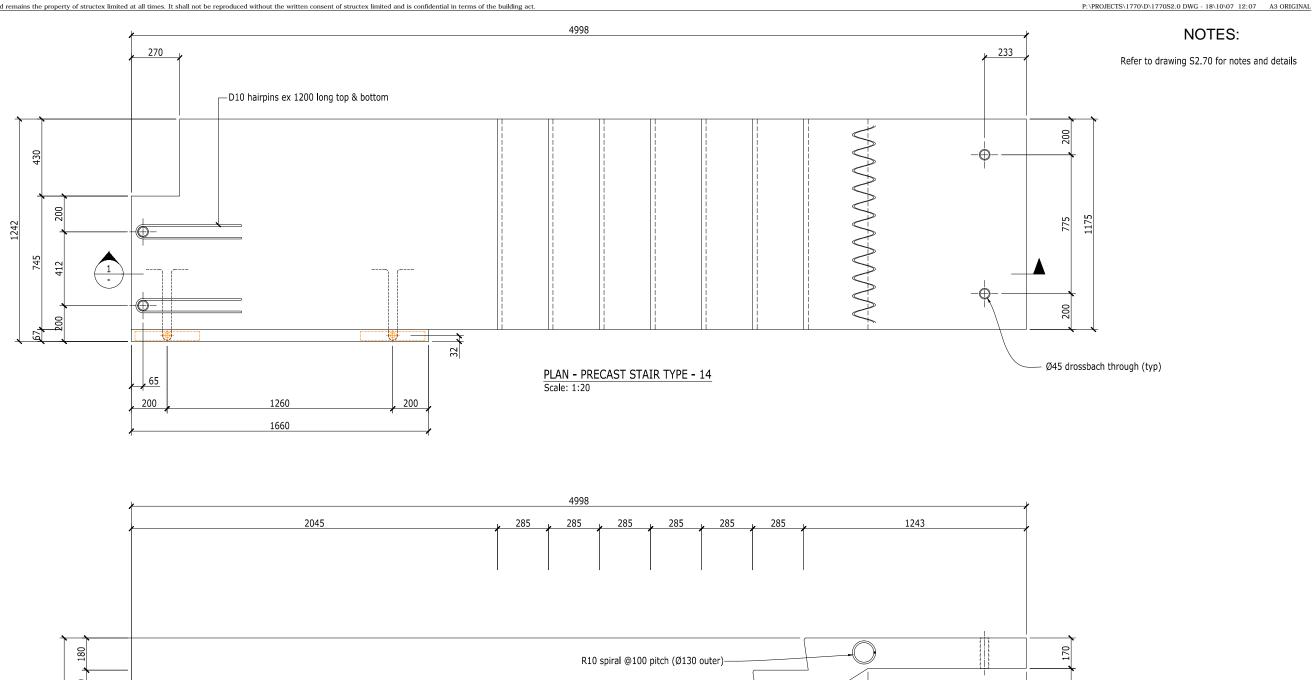


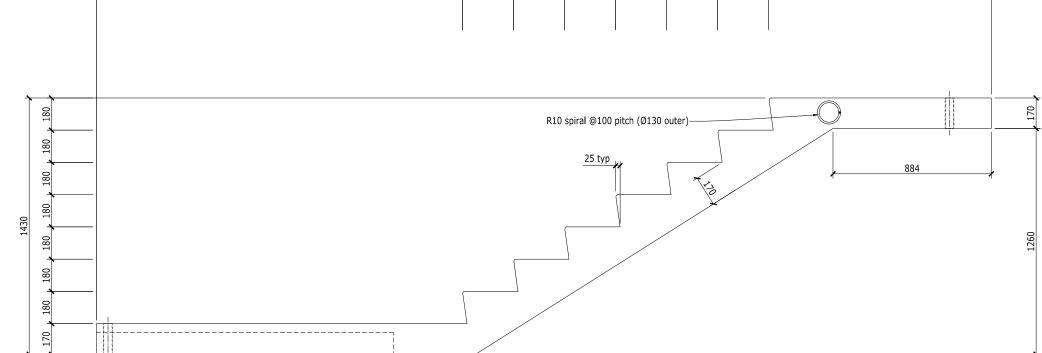












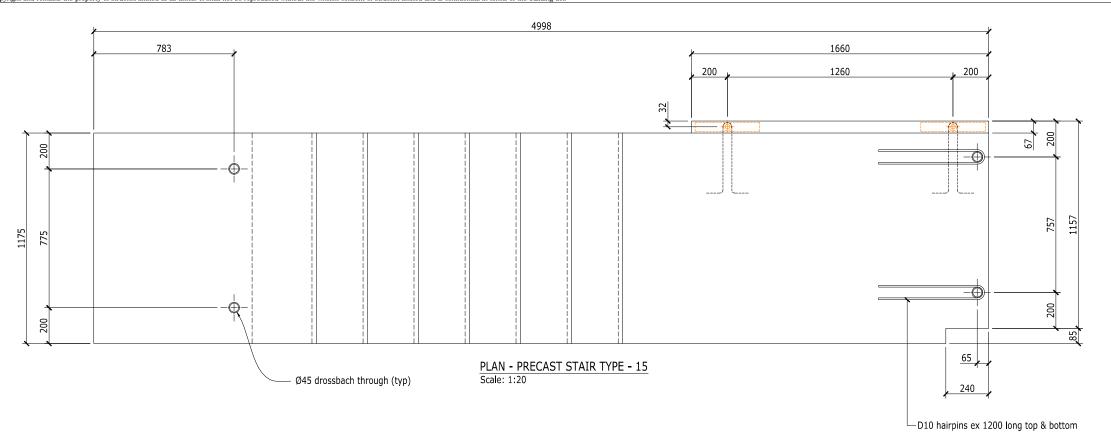


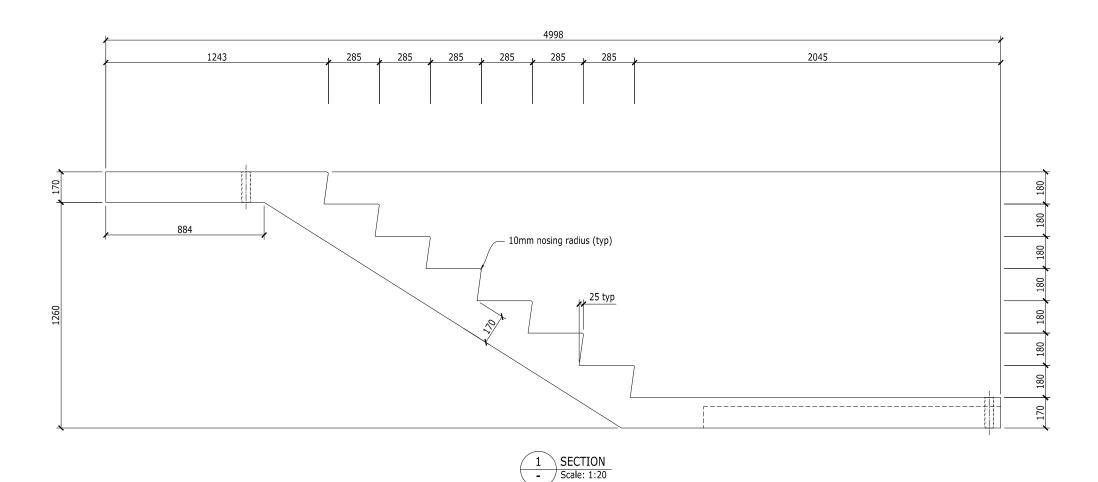






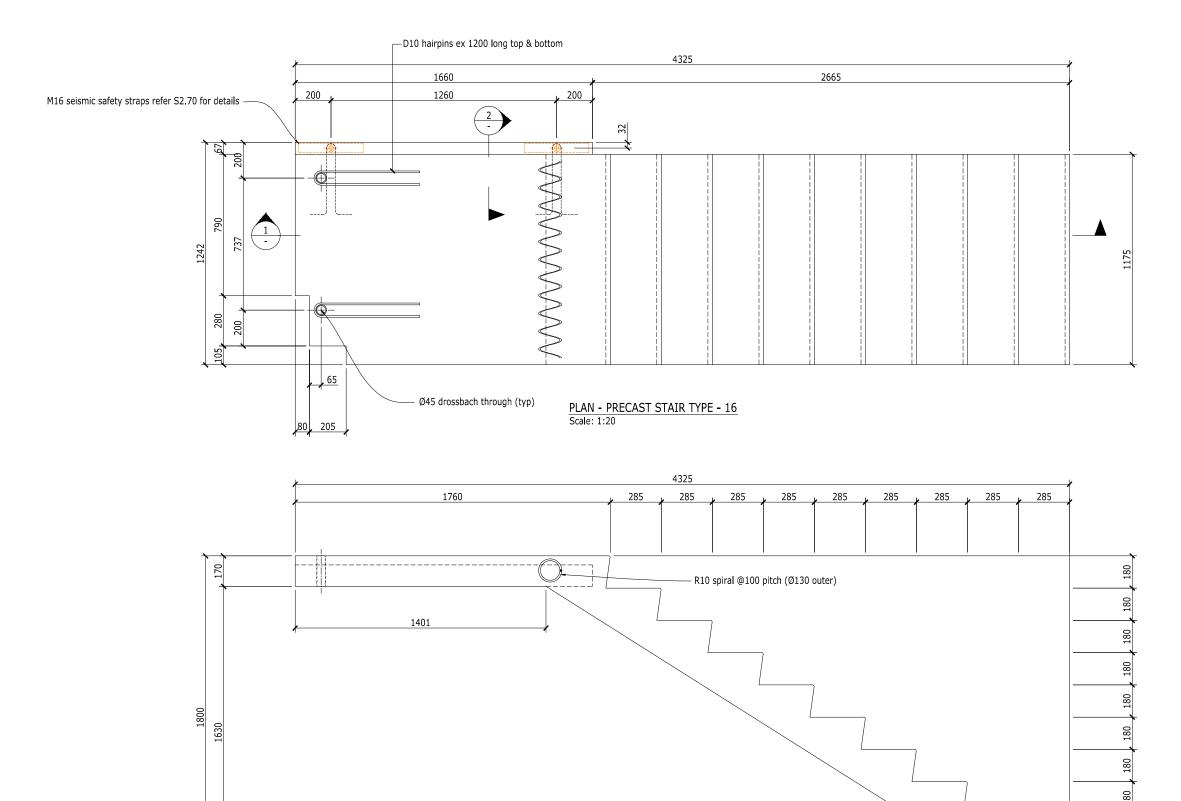
Refer to drawing S2.70 for notes and details







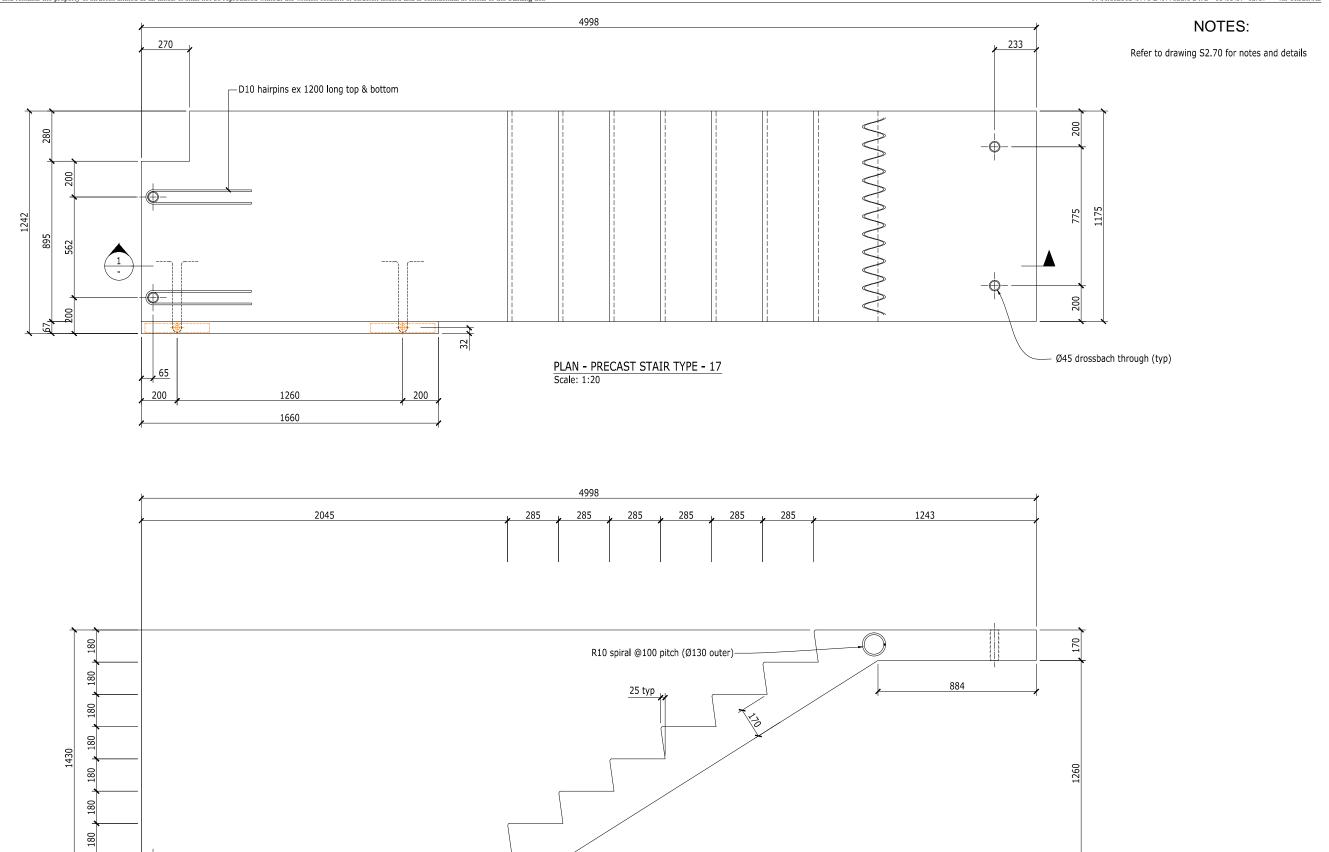
Refer to drawing S2.70 for notes and details











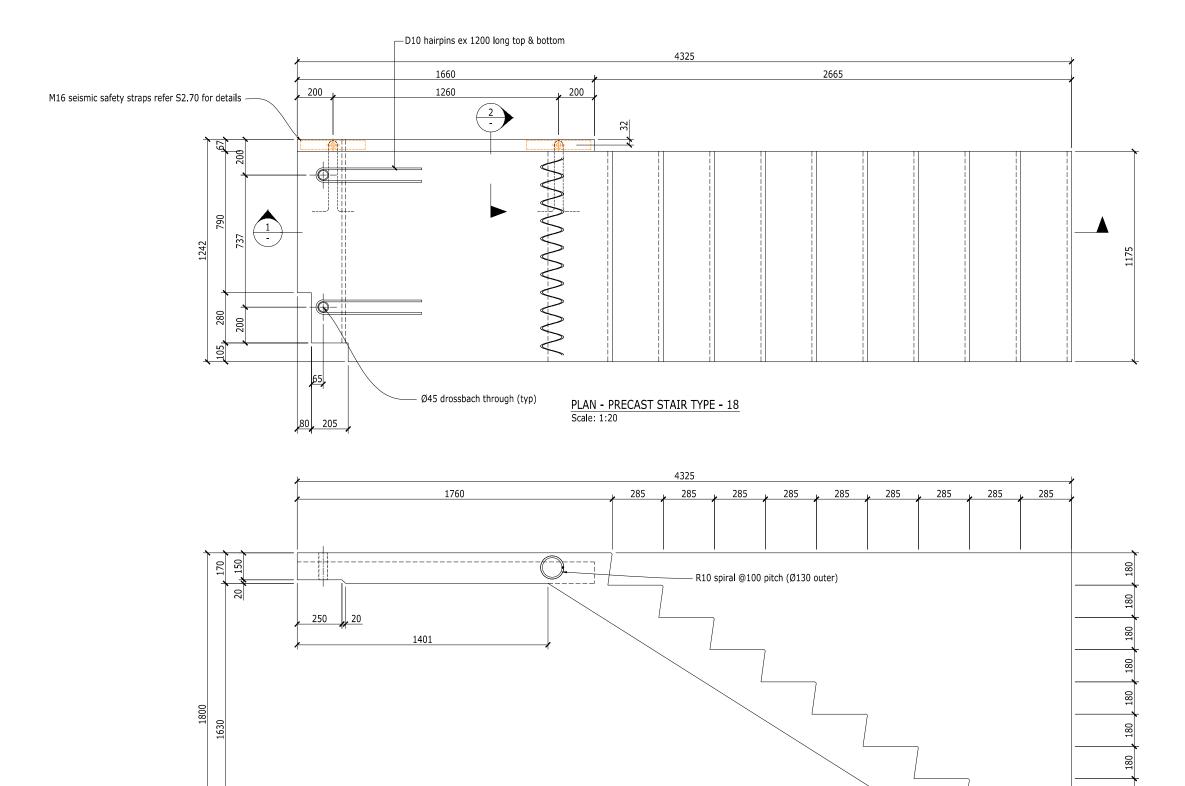








Refer to drawing S2.70 for notes and details



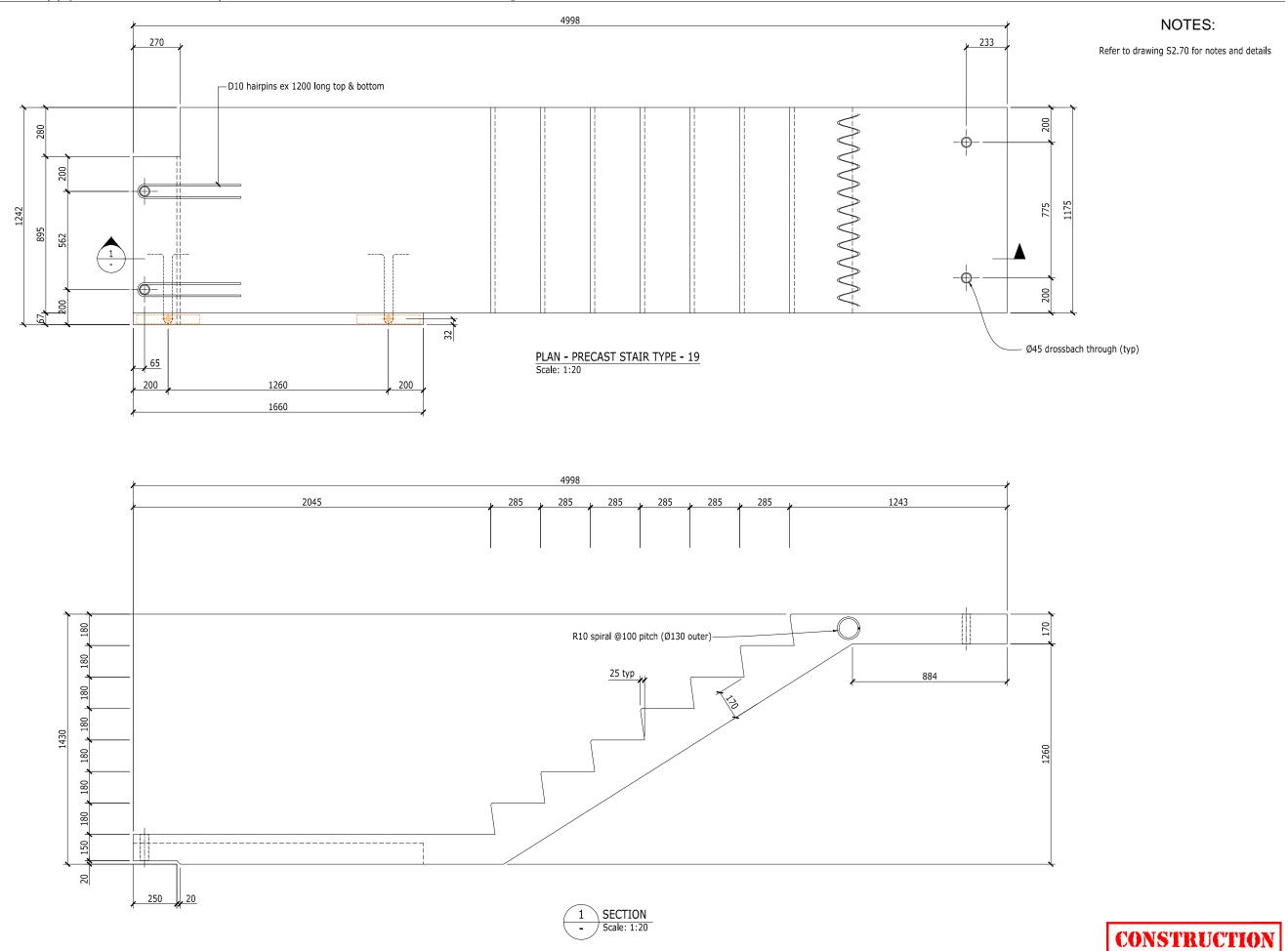








2 CONSTRUCTION ISSUE



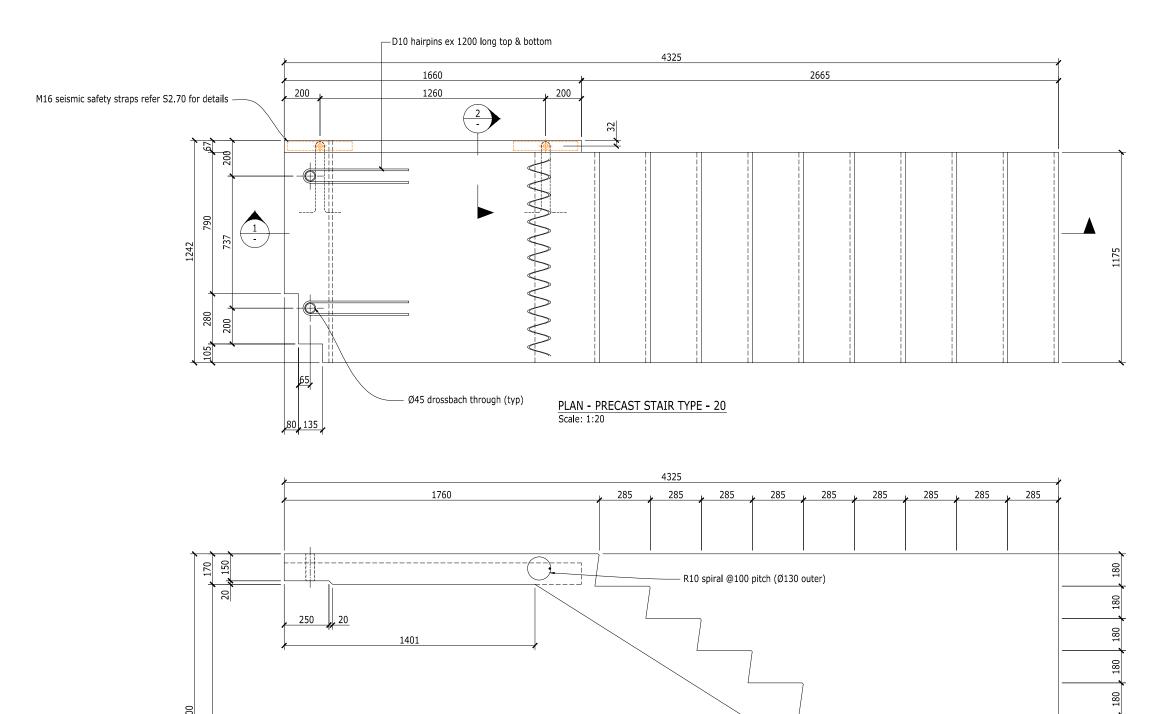
1:20 GA GB C1 TOWER AMC CONSTRUCTION JL SG 12-12-07 by appd. date



PRECAST STAIR TYPE 19

1770 S2.89

Refer to drawing S2.70 for notes and details



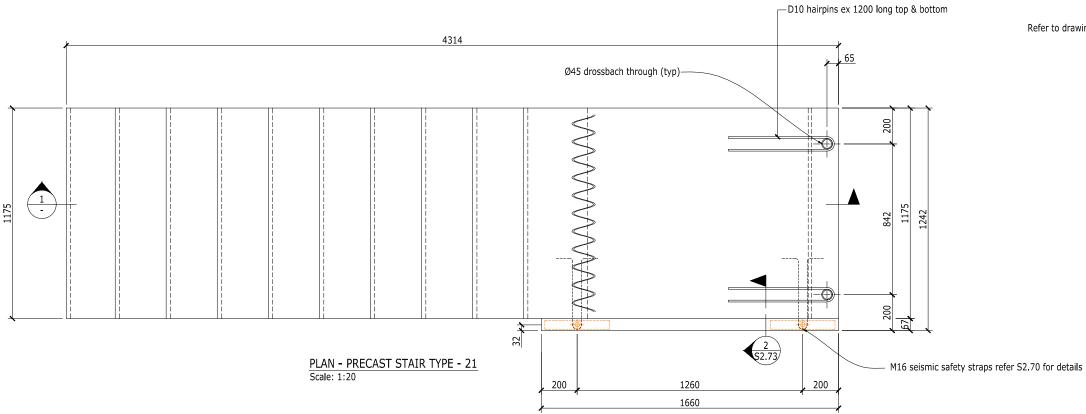


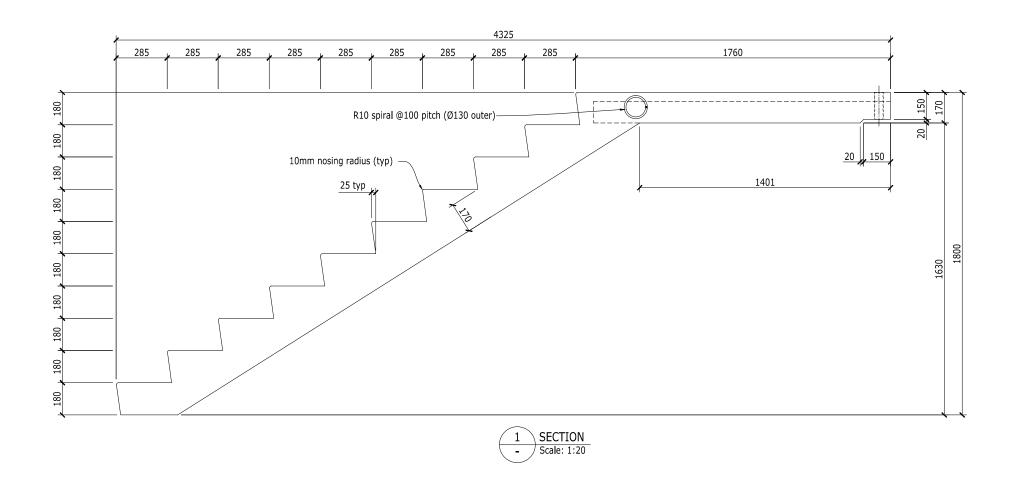
	CONSTRUCTION
#	project





Refer to drawing S2.70 for notes and details











1:20

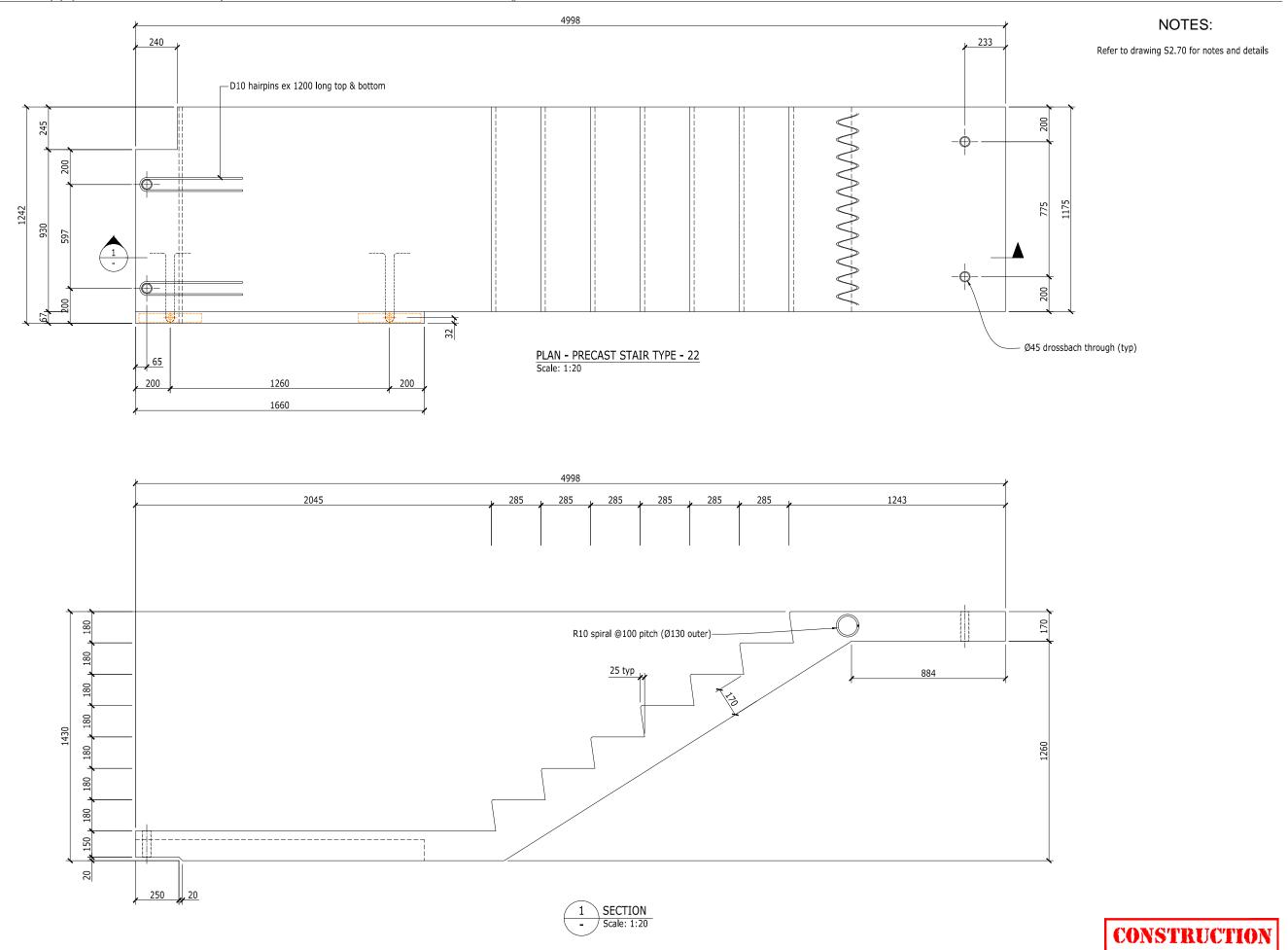
AMC CONSTRUCTION

GA

JL SG 12-12-07 by appd. date

2 CONSTRUCTION ISSUE

GB

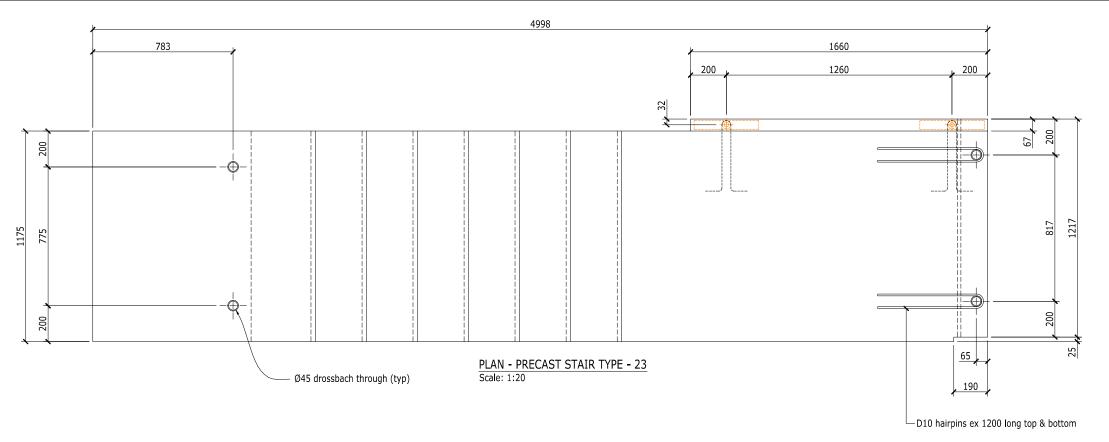


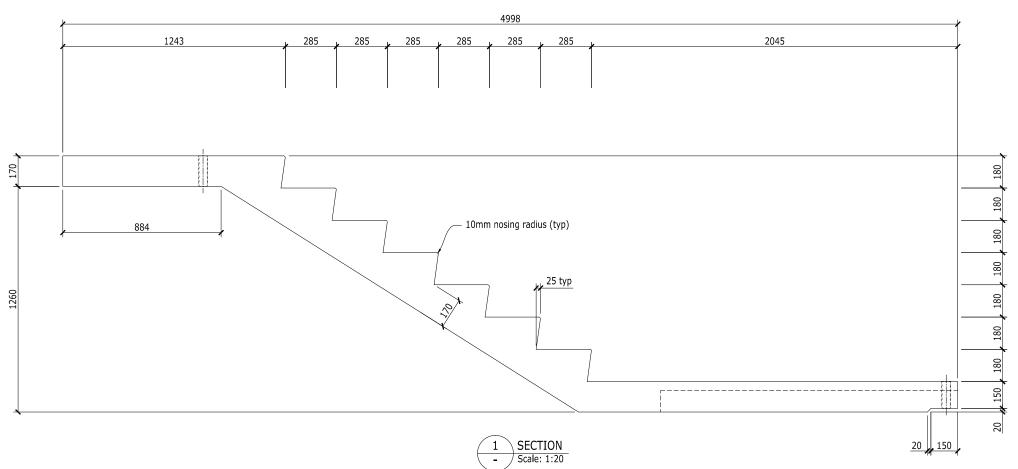
C1 TOWER



PRECAST STAIR TYPE 22

Refer to drawing S2.70 for notes and details



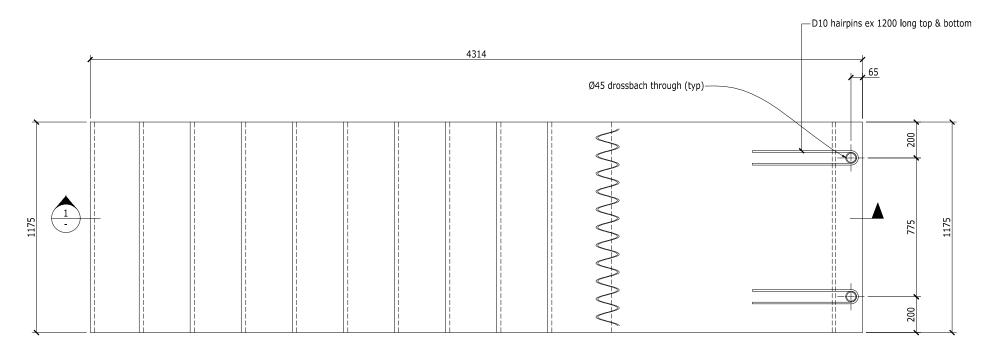


CONSTRUCTION **structex** 

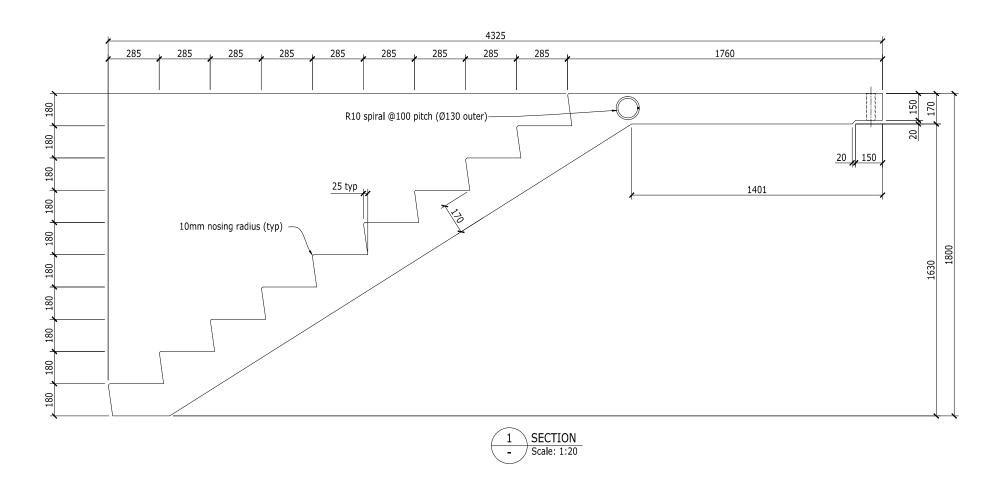
JL SG 12-12-07 by appd. date

1:20

Refer to drawing S2.70 for notes and details



PLAN - PRECAST STAIR TYPE - 24 Scale: 1:20





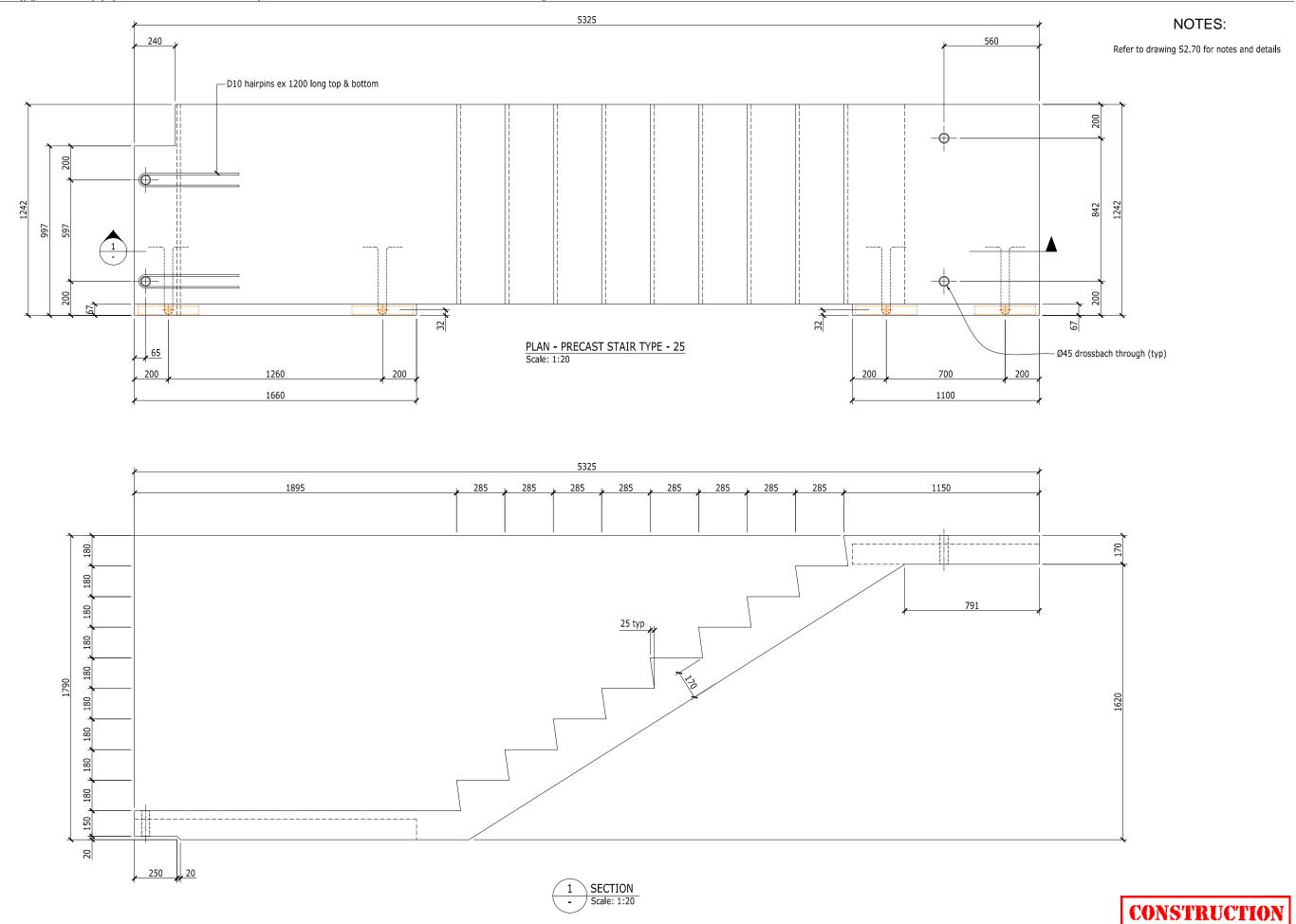
AMC CONSTRUCTION

1:20

GA

GB

C1 TOWER

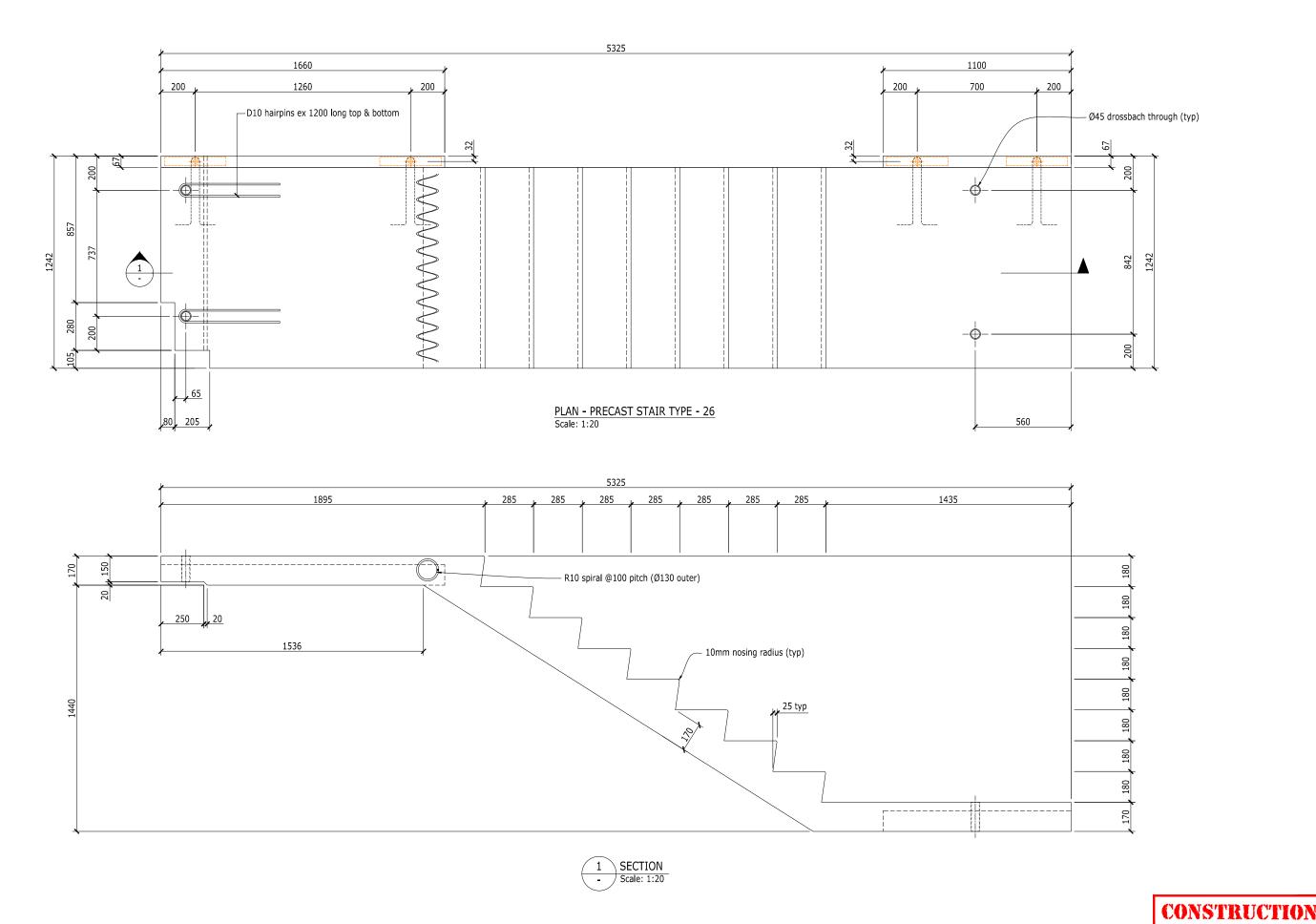


1:20 GA GB C1 TOWER AMC CONSTRUCTION JL SG 12-12-07 by appd. date 2 CONSTRUCTION ISSUE

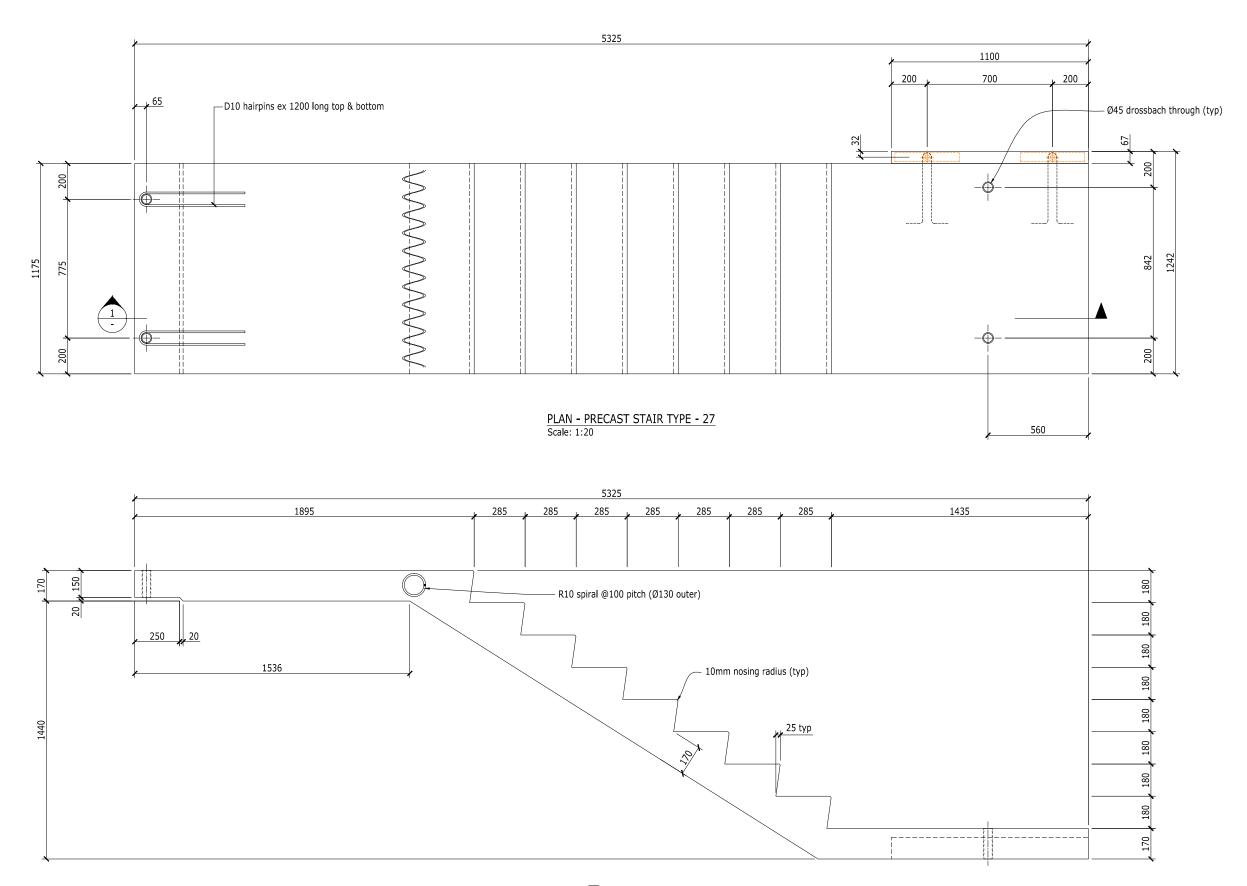




2 CONSTRUCTION ISSUE







SECTION Scale: 1:20

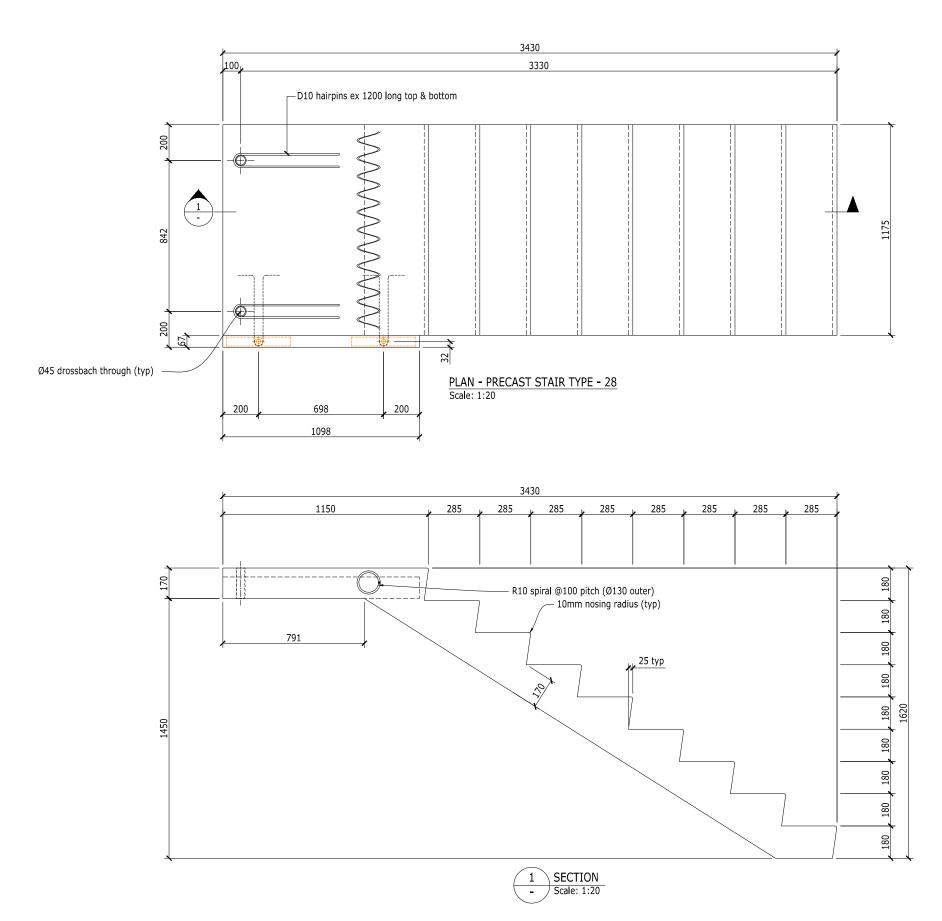




1:20 GA GB AMC CONSTRUCTION JL SG 12-12-07 by appd. date 2 CONSTRUCTION ISSUE

C1 TOWER

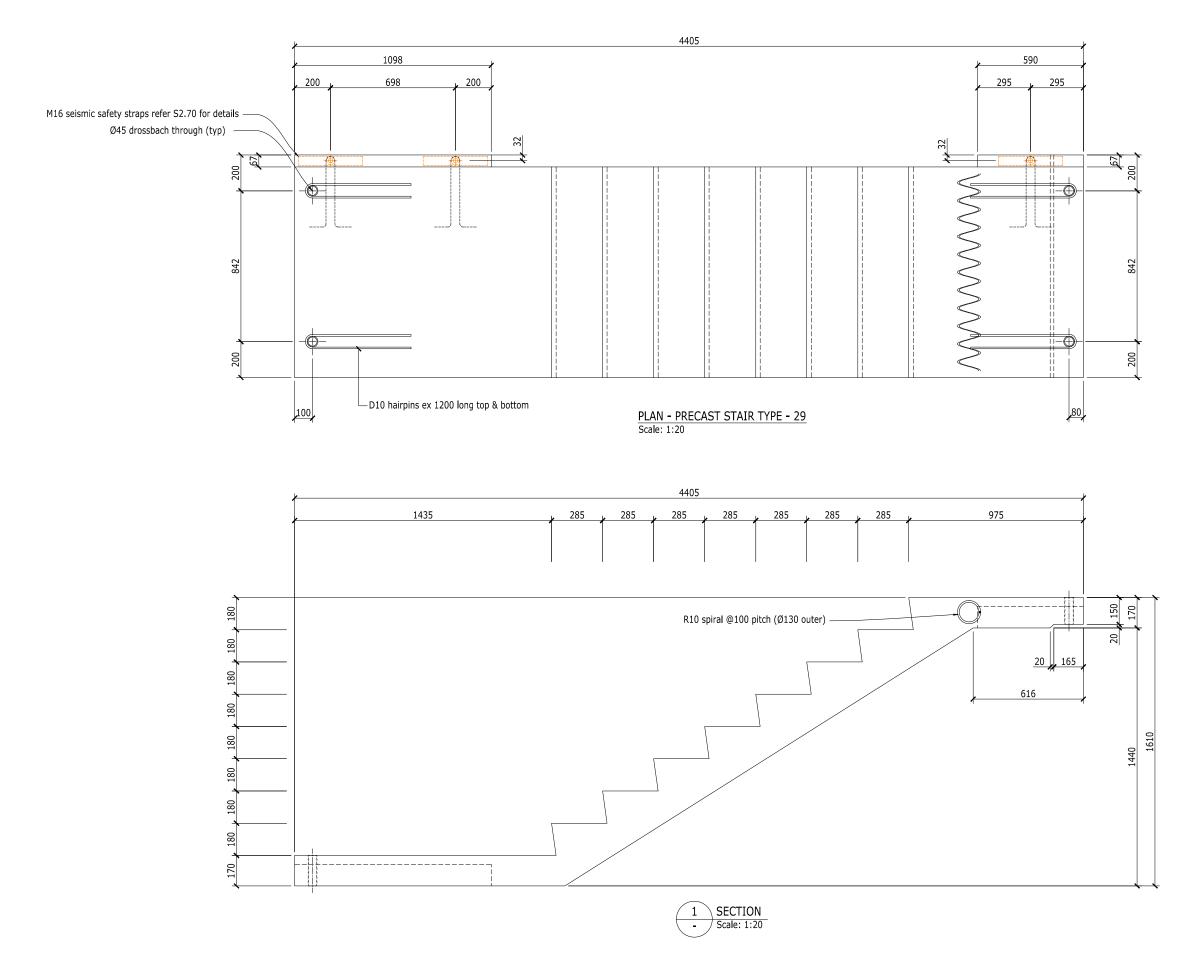
PRECAST STAIR TYPE 27







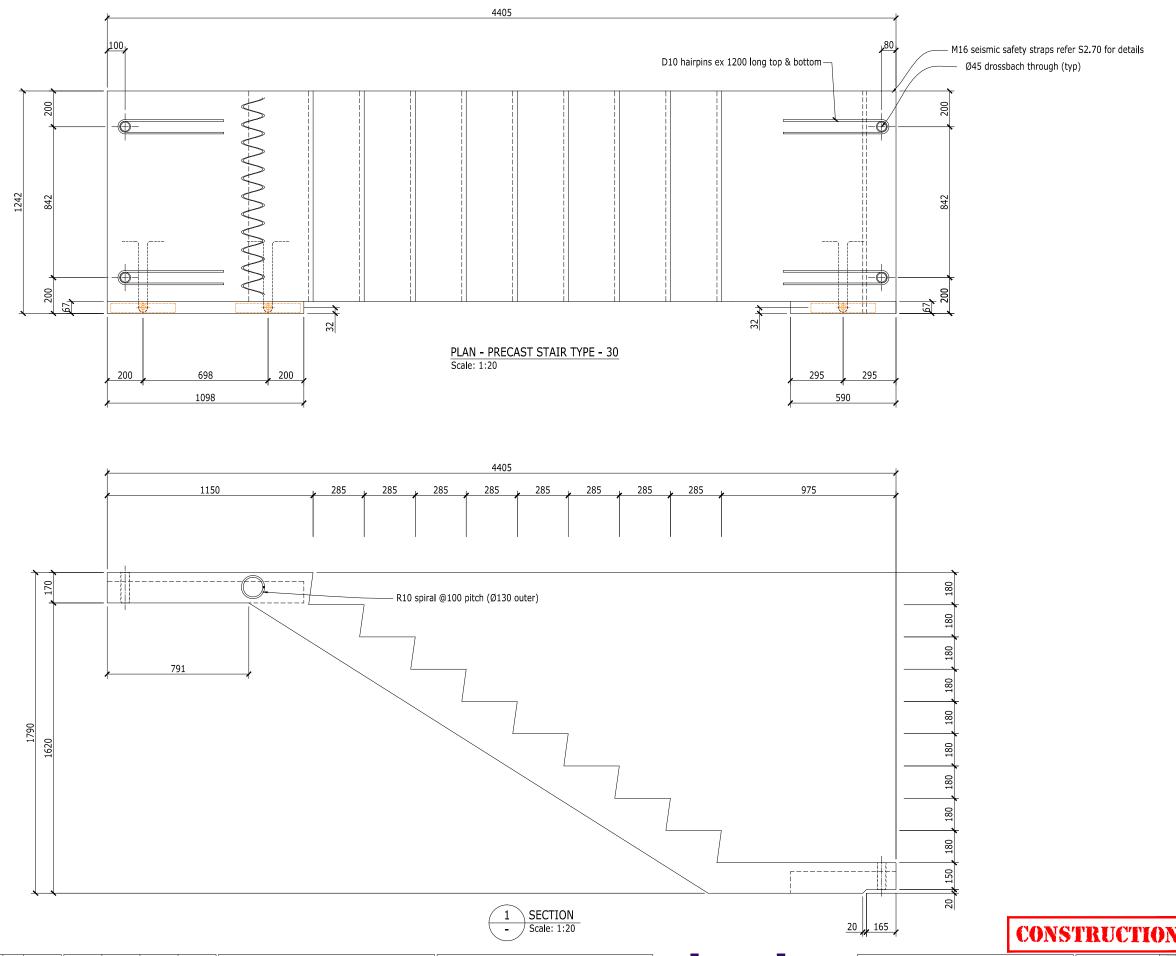




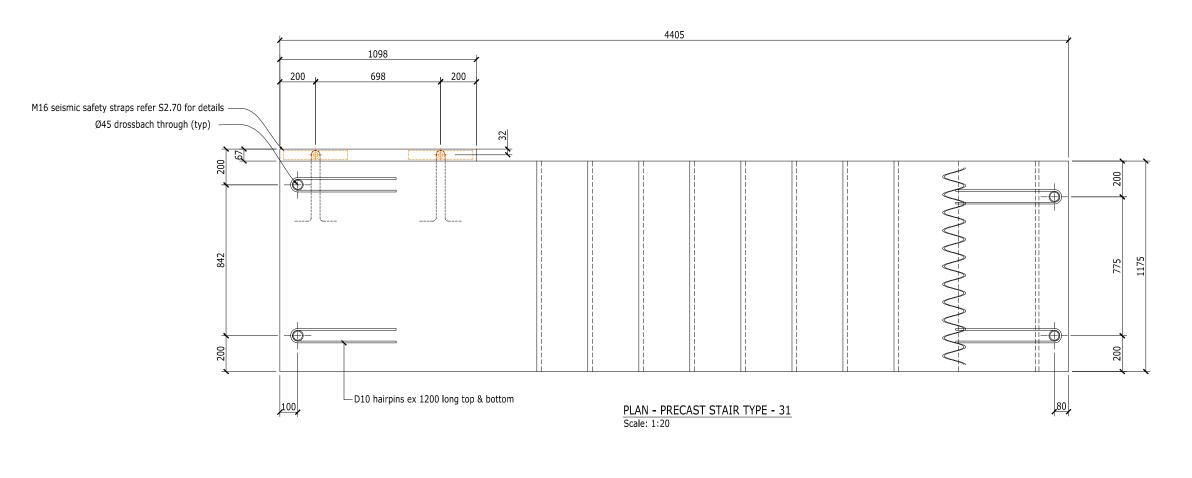


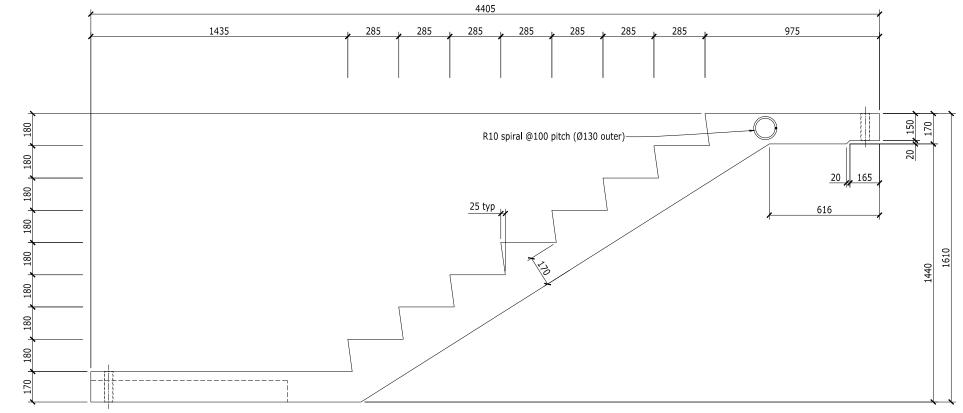


C1 TOWER







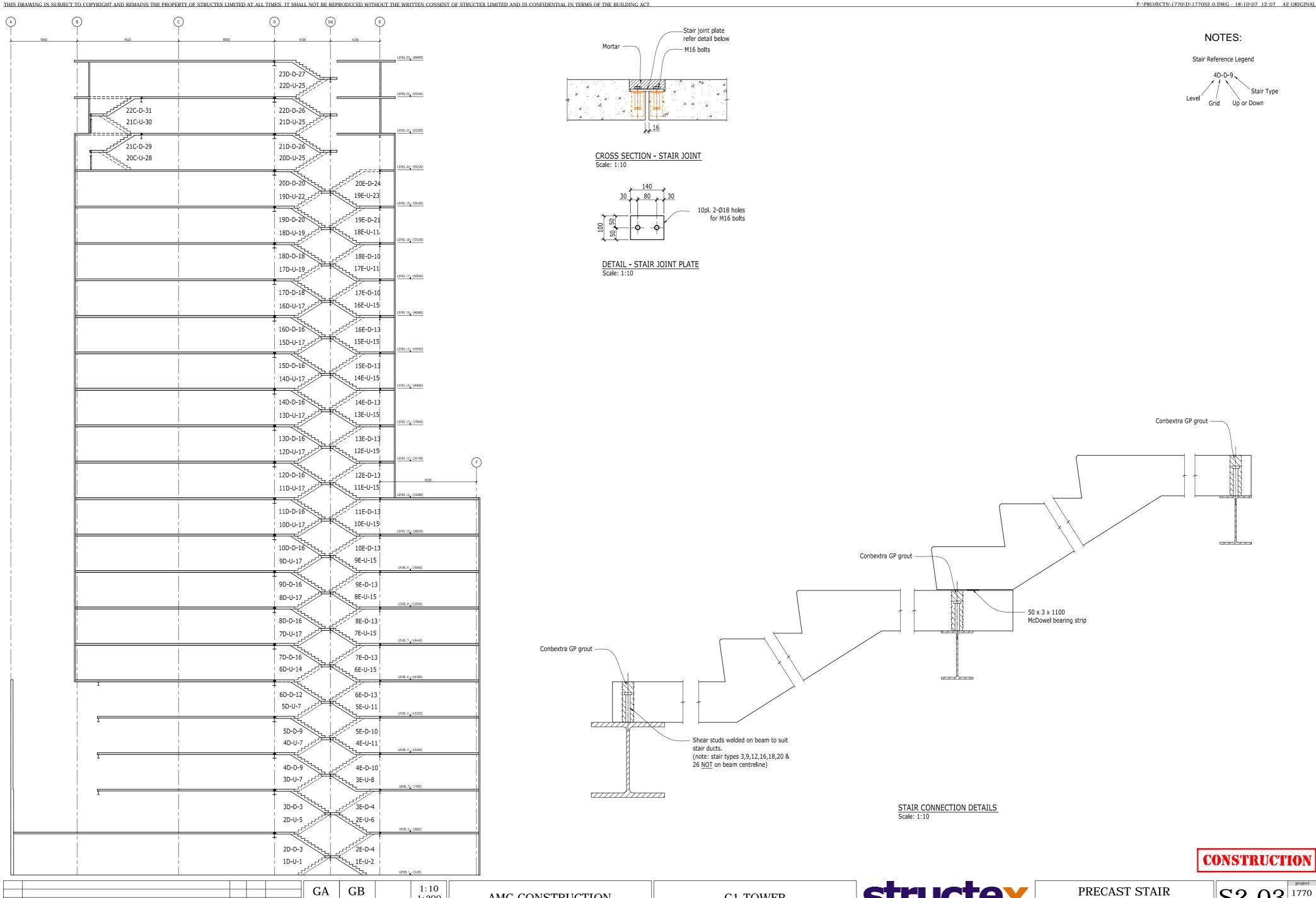












AMC CONSTRUCTION

1:200

JL SG 12-12-07 by appd. date

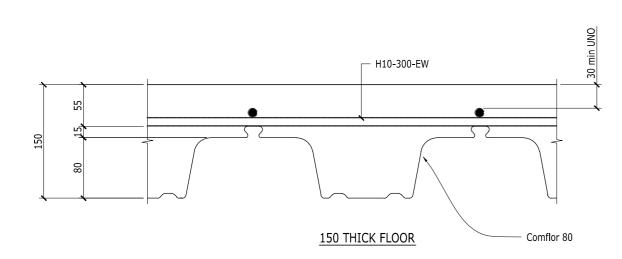
2 CONSTRUCTION ISSUE

C1 TOWER

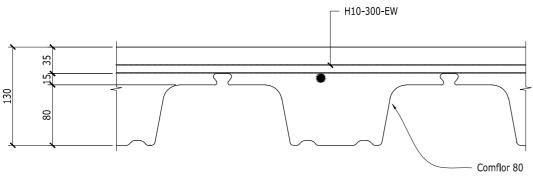
P:\PROJECTS\1770\D\1770S3.0.DWG - 25\10\07 17:59 A2 ORIGINAL

# NOTES:

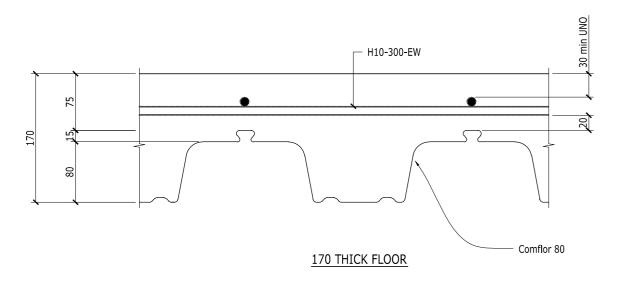
- a. Typical suspended floor reinforcing to be H10 at 300 centres each way
- b. Trimmers to slab wedges to be 4-H12 unless detailed otherwise
- c. Refer Steelwork drawings for layout of comflor 80 system



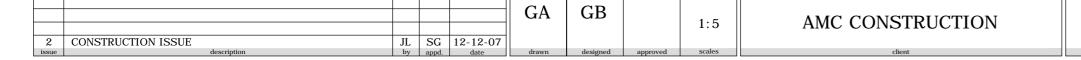
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130 THICK FLOOR - LOCALISED AREAS ONLY



TYPICAL SUSPENDED FLOOR REINFORCEMENT Scale: 1:5



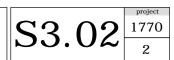


P:\PROJECTS\1770\D\1770S3.0.DWG - 25\10\07 17:59 A2 ORIGINAL THIS DRAWING IS SUBJECT TO COPYRIGHT AND REMAINS THE PROPERTY OF STRUCTEX LIMITED AT ALL TIMES, IT SHALL NOT BE REPRODUCED WITHOUT THE WRITTEN CONSENT OF STRUCTEX LIMITED AND IS CONFIDENTIAL IN TERMS OF THE BUILDING ACT. NOTES: Refer to drawing S3.00 for suspended floor notes & general details 8520 8050 8850 8105 (Dd) 4700 4150 2-H12 trimmers (typ) — 7200 2-H12 trimmers (typ) – 7200 2-H12 trimmers (typ) 4-H12 drag bars 4-H12 drag bars 2 2-H12 trimmers (typ) -- [0]-3 5950-H12 drag bars - 2-RB20 2-H12 trimmers (typ) -—2-H12 trimmers (typ) trim with H16 – 4-RB12 starters from 2-H12 1050 2000 1600 450 4-H12 drag bars stair landings (typ) - 4-H12 drag bars (4) Ø600 & Ø400 holes 4-H12 trimmers (stagger) (stagger) -4 53.31 Load Transfer Tie Plate refer detail sheet S3.31 (6) <sup>1</sup>2-H12 └2-H12 4-H12 drag bars 4-H12 drag bars 6-H16 drag bars— 2-H12 trimmers (typ) -Floor System 150 o/a thick concrete slab on Comflor 80 — 4-H12 drag bars SUSPENDED FLOOR RAMP 2-RB20 3 \$3.31 5 \$3.31 2-H12 trimmers (typ) 2-H12 trimmers (typ)  $\Box$ 4-H12 drag bars 2-H12 trimmers (typ) -

CONSTRUCTION ISSUE J	I.	SG 12-12-07	GA	GB		1:100	AMC CONSTRUCTION	C1 TOWER	structex
	by	appd. date	drawn	designed	approved	scales	client	project title	"Giving support a whole new meaning" www.structex.co.nz







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NOTES: Refer to drawing S3.00 for suspended floor notes & general details 8520 8105 (Dd) 4700 4150 4-H12 drag bars 2-H12 trimmers (typ) -(3) 4-H12 drag bars 4-H12 drag bars (ex 12000 long) 2-H12 trimmers (typ) 4 Load Transfer Tie Plates 4-H12 drag bars 2-H12 - 4-H12 drag bars -H12 drag bars refer detail sheet S3.31 -4-H12 drag bars (9000 long, stagger) (6) 3-D12 (typ) — 4-H12 drag bars 2-H12 trimmers (typ) -— 4-RB12 starters from stair landings (typ) 2-H12 trimmers — 2-RB20 — 2-RB20 4-H12 drag bars \_\_\_\_ B \$3.33 2-H12 trimmers (typ) — 4-H12 drag bars

CONSTRUCTION







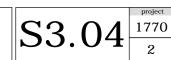
NOTES: Refer to drawing S3.00 for suspended floor notes & general details 8850 8520 8050 8105 (Dd) 4700 4150 (2) 4-H12 drag bars 2-H12 trimmers (typ) -3 4-H12 drag bars --4-H12 drag bars Floor System 150 o/a thick concrete slab on Comflor 80 - 2-H12 trimmers (typ) 4 Load Transfer Tie Plates refer detail sheet S3.31 — -4-H12 drag bars 2-H12 (9000 long (stagger) 2 53.31 4-H12 drag bars (9000 long, stagger) — 2-H12-Canterlever 150mm slap (6) 3-D12 (typ) 9 53.31 -4-H12 drag bars 2-H12 trimmers — 4-RB12 starters from stair landings (typ) \_\_\_\_ 2-H12 trimmers - 2**-**RB20 – 2**-**RB20 B \$3.33 2-H12 trimmers (typ) -4-H12 drag bars —

CONSTRUCTION









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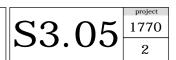
NOTES: Refer to drawing S3.00 for suspended floor notes & general details 8520 8850 8105 (Dd) 4700 4150 (2) 4-H12 drag bars 2-H12 trimmers (typ) (3) 4-H12 drag bars 4-H12 drag bars Floor System 150 o/a thick concrete slab - 2-H12 trimmers (typ) on Comflor 80 2-H12 1425 4 Load Transfer Tie Plates 4-H12 drag bars - 4-H12 drag bars refer detail sheet S3.31 -2-H12 (9000 long (stagger) 2 \$3.31 4-H12 drag bars (9000 long, stagger) Canterlever Floor (6) 3-D12 (typ) 9 (53.31) 4-H12 drag bars 2-H12 trimmers — 4-RB12 starters from stair landings (typ) 2-H12 trimmers 4-H12 drag bars – 2**-**RB20 - 2**-**RB20 (ex 12000 long) B \$3.33 4-H12 drag bars 2-H12 trimmers (typ) -

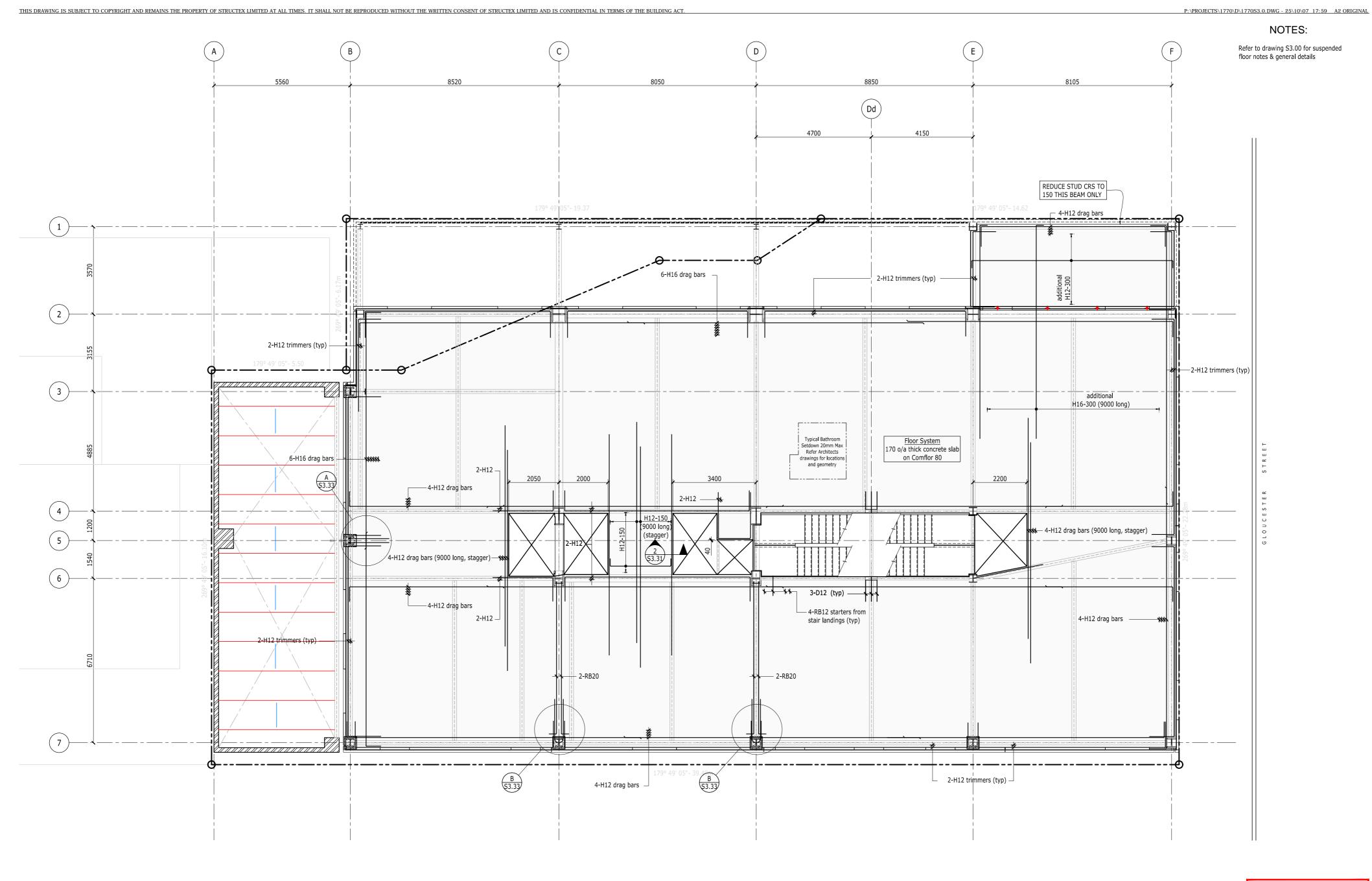
CONSTRUCTION



















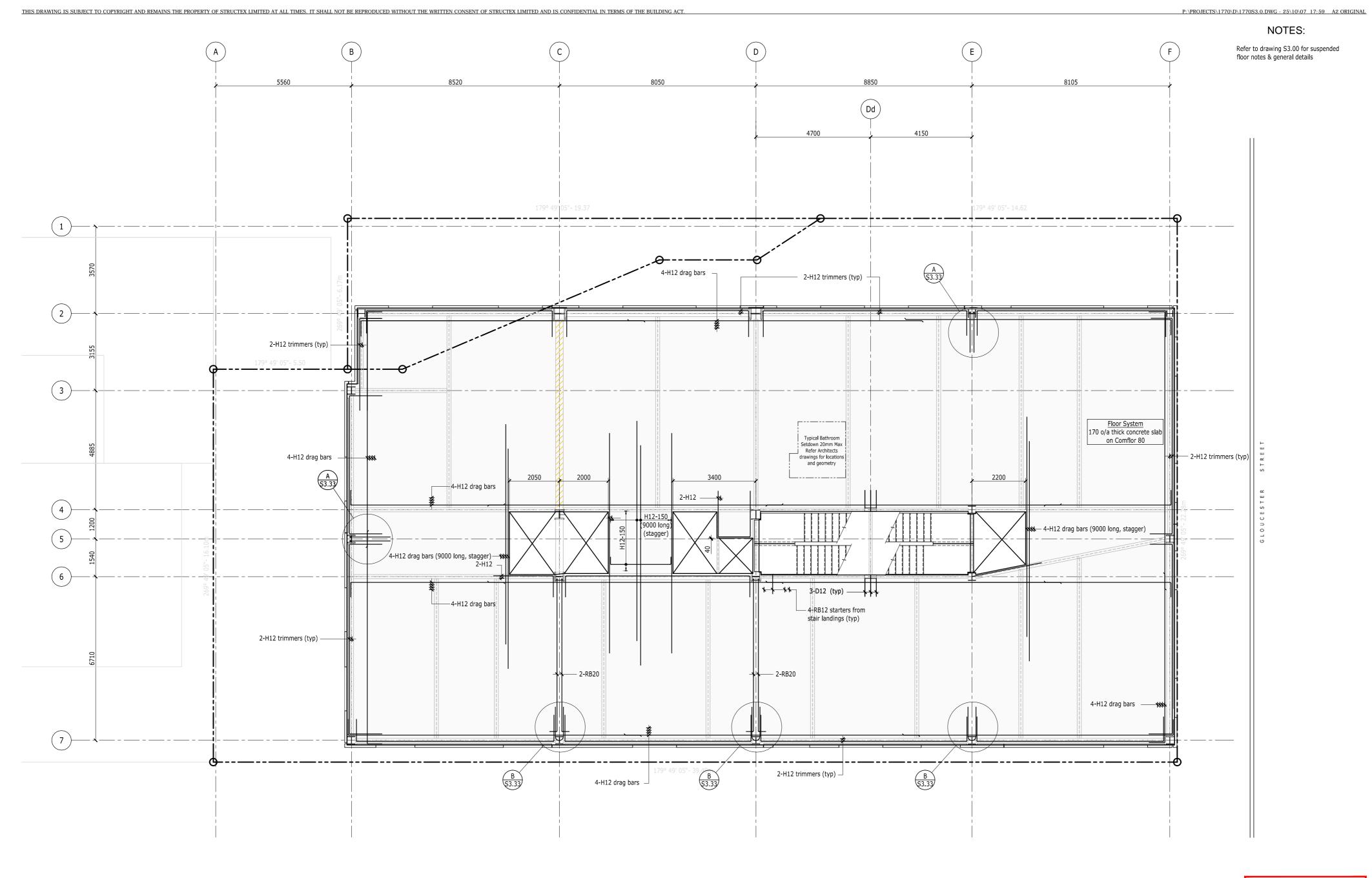
NOTES: Refer to drawing S3.00 for suspended floor notes & general details 8520 8050 8850 8105 (Dd) 4700 4150 179° 49' 05"- 19.37 A \$3.33 4-H12 drag bars 2-H12 trimmers (typ) 2 2-H12 trimmers (typ) -(3) Floor System 170 o/a thick concrete slab Typical Bathroom
Setdown 20mm Max
Refer Architects
drawings for locations
and geometry on Comflor 80 — 2-H12 trimmers (typ) 4-H12 drag bars A \$3.33 2200 -4-H12 drag bars 2-H12 — (9000 long - 4-H12 drag bars (9000 long, stagger) (stagger) 4-H12 drag bars (9000 long, stagger) — 2-H12 -(6) 3-D12 (typ) – 4-H12 drag bars - 4-RB12 starters from stair landings (typ) 2-H12 trimmers (typ) -- 2-RB20 - 2**-**RB20 B \$3.33 2-H12 trimmers (typ) -

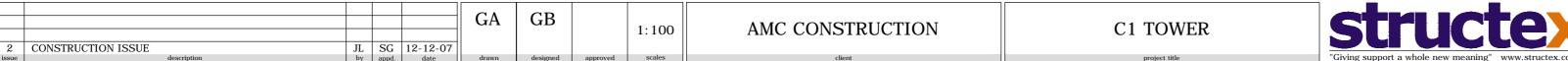
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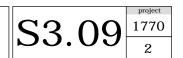












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Setdown 20mm Max
Refer Architects
drawings for locations
and geometry on Comflor 80 4-H12 drag bars — 2-H12 trimmers (typ) A \$3,33 2200 4-H12 drag bars 2<del>-</del>H12 – (4) (9000 long) - 4-H12 drag bars (9000 long, stagger) (stagger) 4-H12 drag bars (9000 long, stagger) — 2-H12 \_ (6) 3-D12 (typ) --4-H12 drag bars − 4-RB12 starters from stair landings (typ) 2-H12 trimmers (typ) -— 2-RB20 | — 2-RB20 4-H12 drag bars — B \$3.33 









NOTES: Refer to drawing S3.00 for suspended floor notes & general details 8850 (Dd) 4700 4150 179° 49' 05"- 19.37 4-H12 drag bars 2-H12 trimmers (typ) (2) 2-H12 trimmers (typ) (3) Floor System 170 o/a thick concrete slab Typical Bathroom
Setdown 20mm Max
Refer Architects
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and geometry on Comflor 80 ─ 2-H12 trimmers (typ) 4-H12 drag bars A \$3.33 4-H12 drag bars 2-H12 — – 4-H12 dyrag bars (9000 long, stagger) (9000 long (stagger) 2 - Ø600 Penetrators TYP 4-H12 drag bars (9000 long, stagger)—\*
2-H12 — (6) 3-D12 (typ) — —4-H12 drag bars 4-RB12 starters from stair landings (typ) 2-H12 trimmers (typ) -— 2**-**RB20 — 2-RB20 4-H12 drag bars — B \$3.33 B \$3.33 2-H12 trimmers (typ) 4-H12 drag bars —

CONSTRUCTION

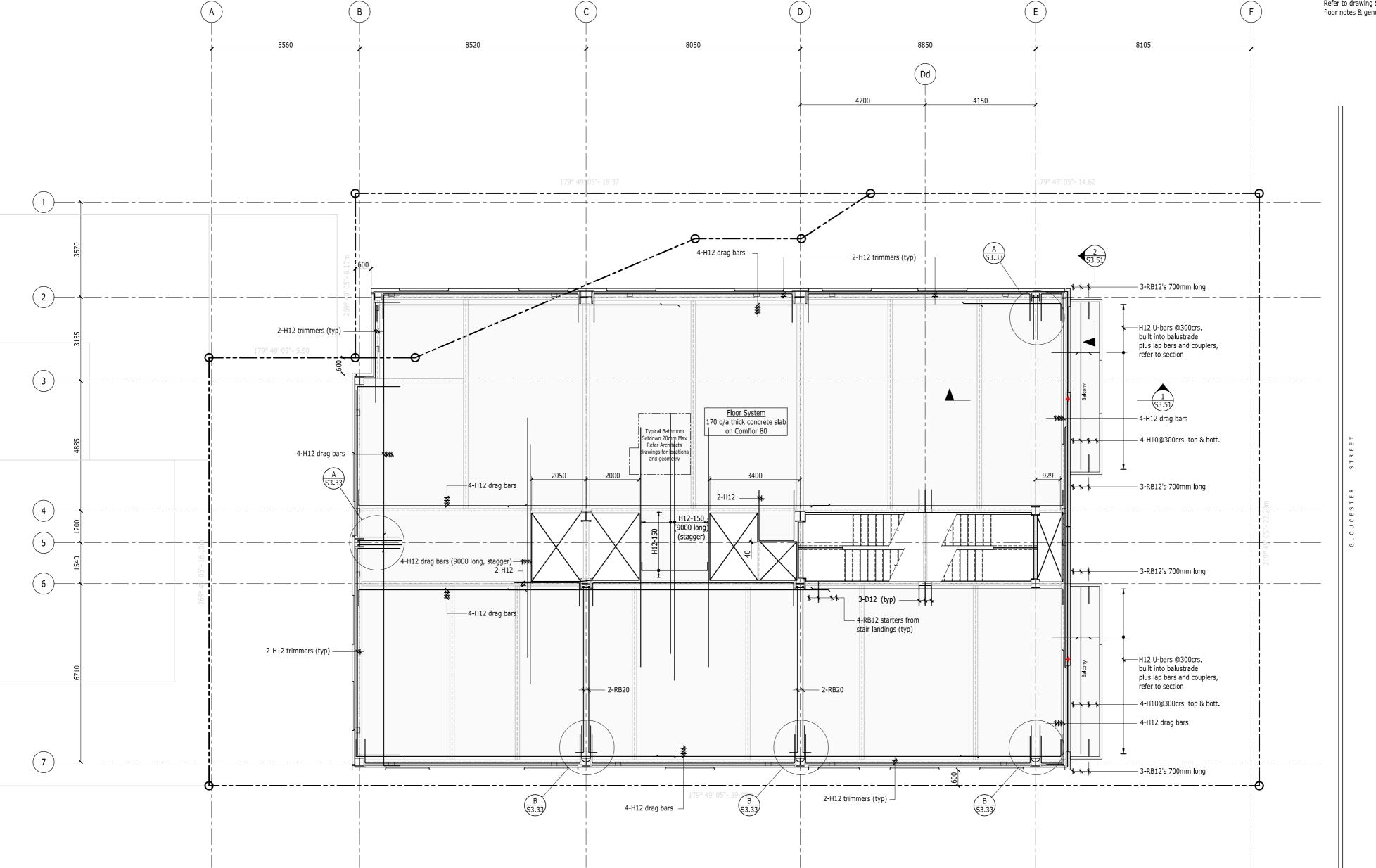








Refer to drawing S3.00 for suspended floor notes & general details





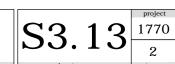


CONSTRUCTION









CONSTRUCTION













CONSTRUCTION





CONSTRUCTION



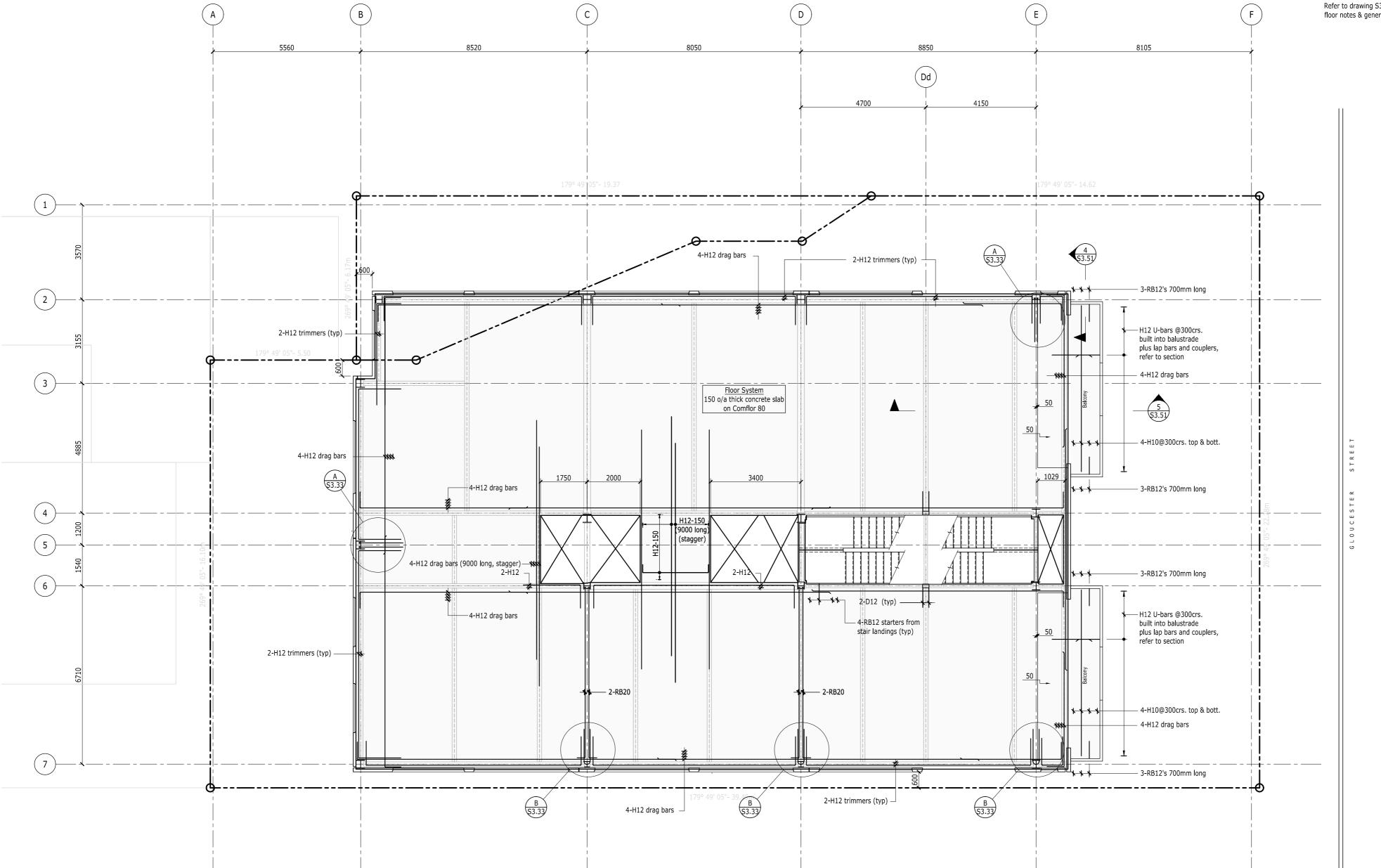




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## NOTES:

Refer to drawing S3.00 for suspended floor notes & general details





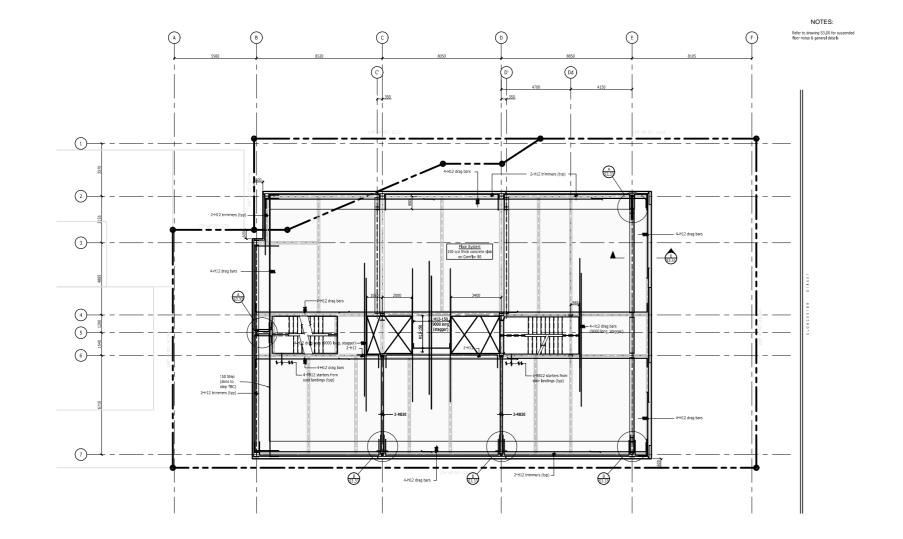


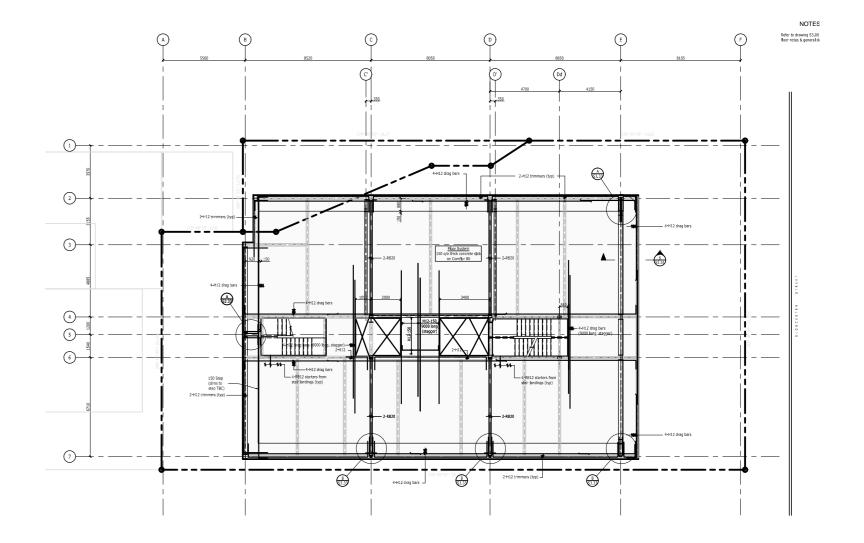
CONSTRUCTION



4-H12 drag bars -







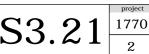








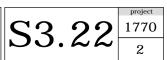




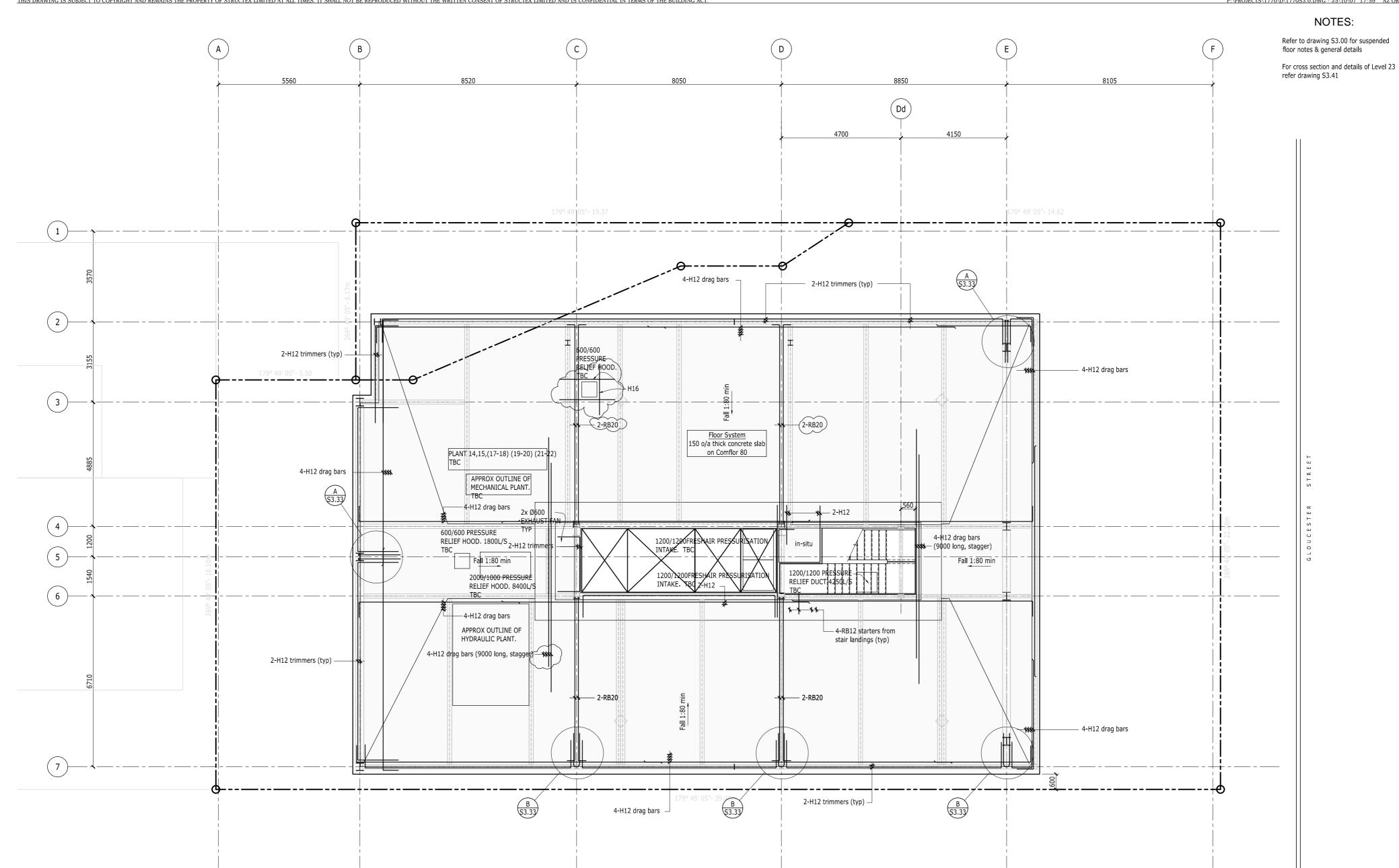








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NOTES: Refer to drawing S3.00 for suspended floor notes & general details For cross section and details of Level 23 refer drawing S3.41 8520 8850 (Dd) 4700 4150 (2) (3) 15 series block walls reinforced with H12 at 400 crs EW UNO Floor System
200 o/a thick concrete slab Fall 1:80 min Fall 1:80 min on Comflor 80 200 thick Comflor 80 reinforced with H12 EW at 400 crs. Cast in penetrations as required by lift manufacturer

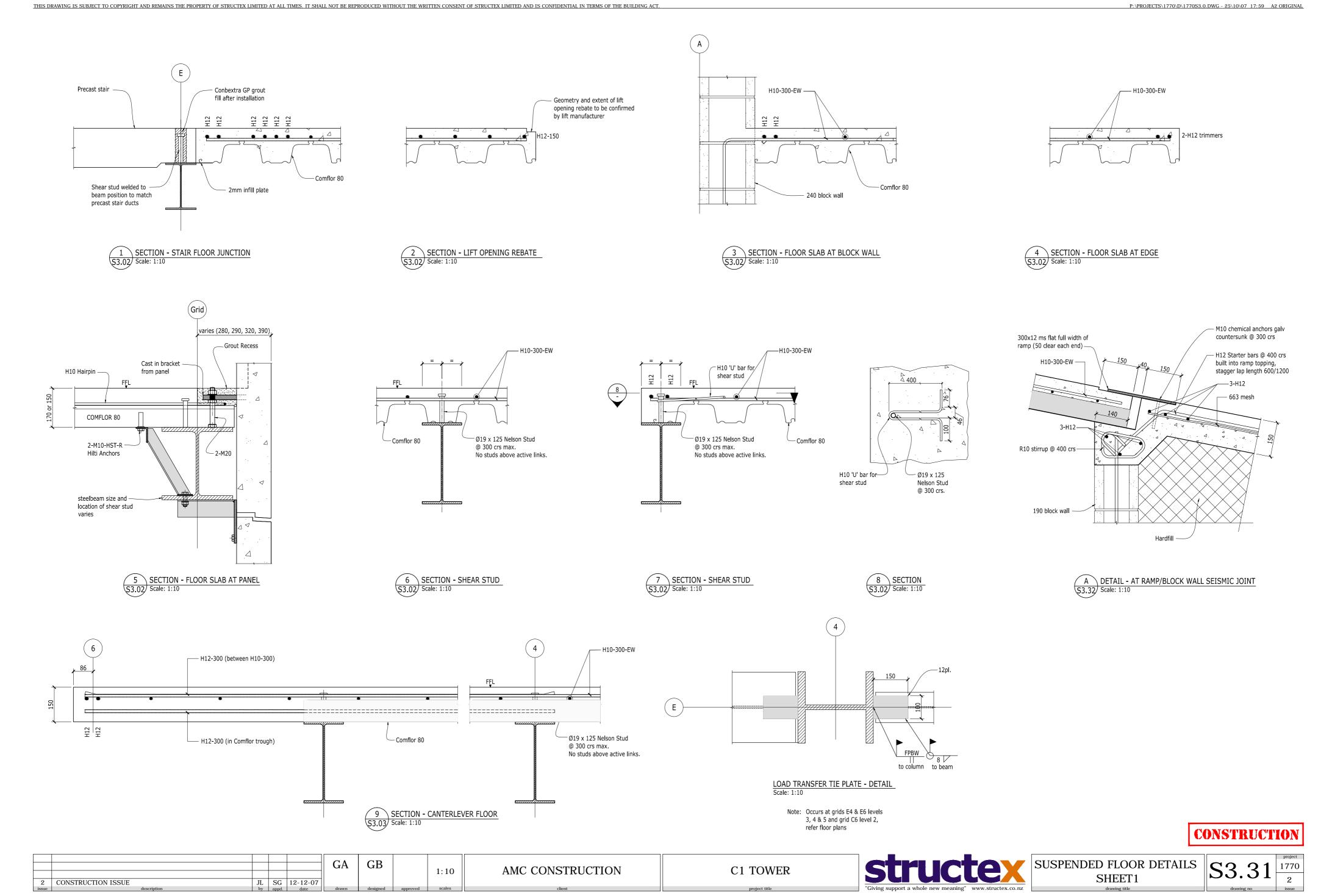
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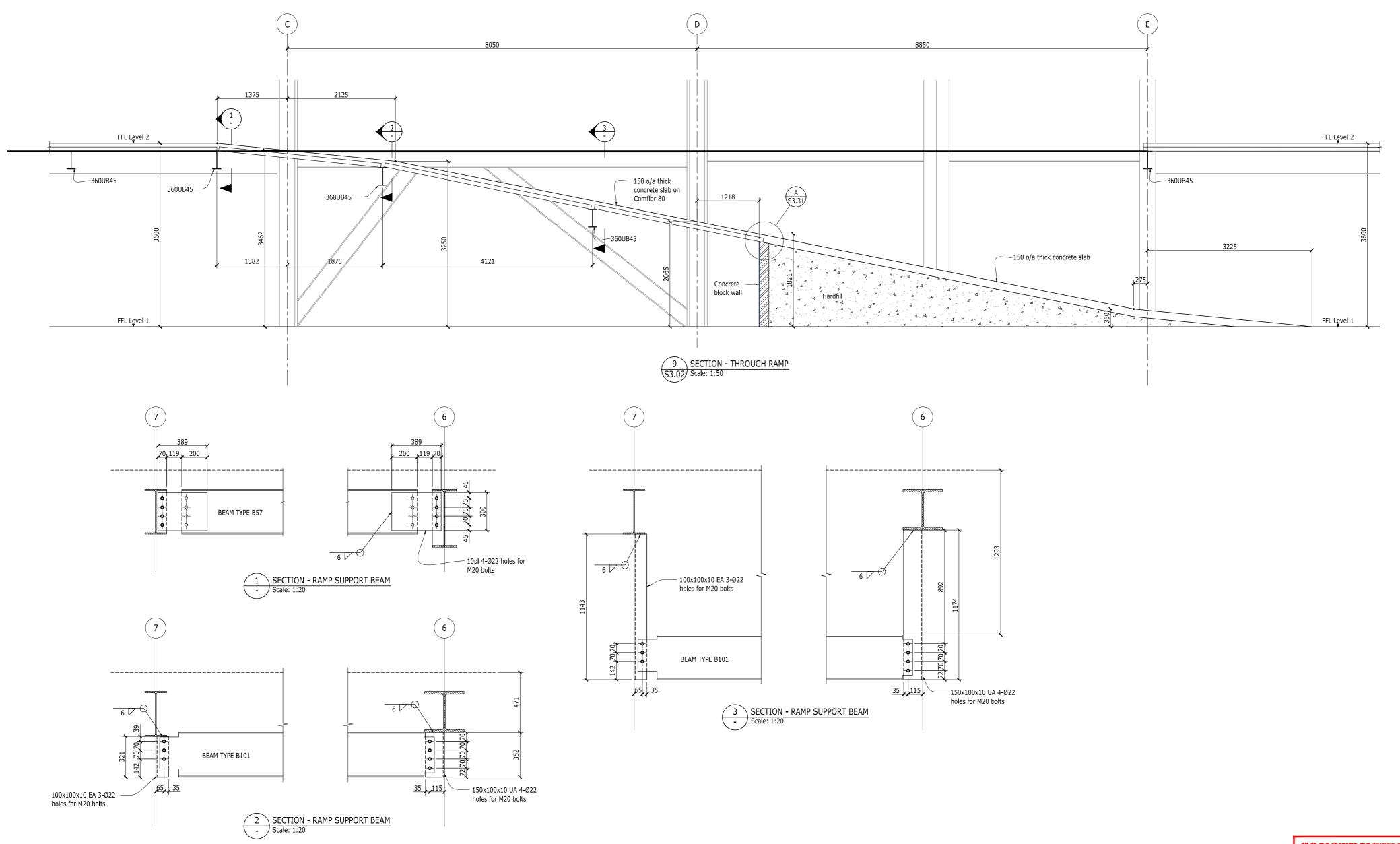


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LEVEL 24 FLOOR PLAN

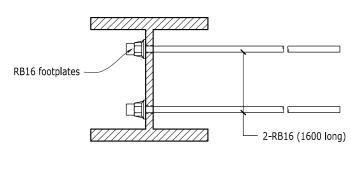


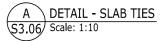


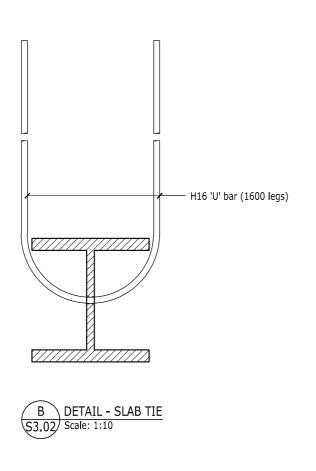
CONSTRUCTION

		GA GB	1:50	AMC CONSTRUCTION	C1 TOWER	structex
CONSTRUCTION ISSUE JL	SG 12-12-07					
description by	annd date	drawn designed	approved scales	client	project title	"Giving support a whole new meaning" www.structex.co.nz

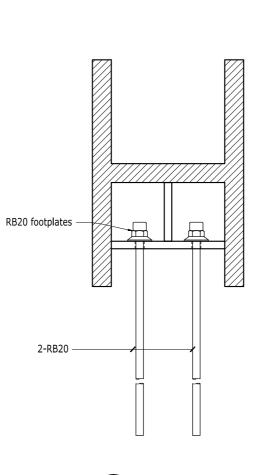








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C DETAIL - SLAB TIES S3.02 Scale: 1:10





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- Scale: 1:10 290 1115 LEVEL 23 —H12-400-EW to slab 310UB46 360UB57 - Comflor 80 15 series concrete block walls LEVĘL 22 C DETAIL - LIFT PLANTROOM FLOOR LEVEL 23 CROSS SECTION BETWEEN GRIDS C & D Scale: 1:50 H12-400 starters to project Note: Block Reinforcement -800 above concrete plinth H12 at 400 centres each way FPBW to steel beams. unless noted otherwise H10 Ubar to each Nelson Stud -H10-300-EW 600 Lap -—H10-300-EW - R6 @ 300 310UB46 \_ Nelson Shear Stud - Central (not above active link) 100x100x10 EA welded to each face of beam web A DETAIL - FLOOR EDGE - Scale: 1:10 B DETAIL - FLOOR EDGE - Scale: 1:10 CONSTRUCTION LEVEL 23 CROSS SECTION

GA GB C1 TOWER AMC CONSTRUCTION 1:10 2 CONSTRUCTION ISSUE JL SG 12-12-07 by appd. date





