

Canterbury Earthquakes Royal Commission HOTEL GRAND CHANCELLOR

Christchurch Earthquake

CBD Building Performance Technical Investigation

Report on the Structural Performance

of the

Hotel Grand Chancellor

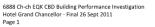
in the

Earthquake of 22 February 2011

Prepared By: Dunning Thornton Consultants Ltd

For: The Department of Building & Housing

Final: 26 September 2011







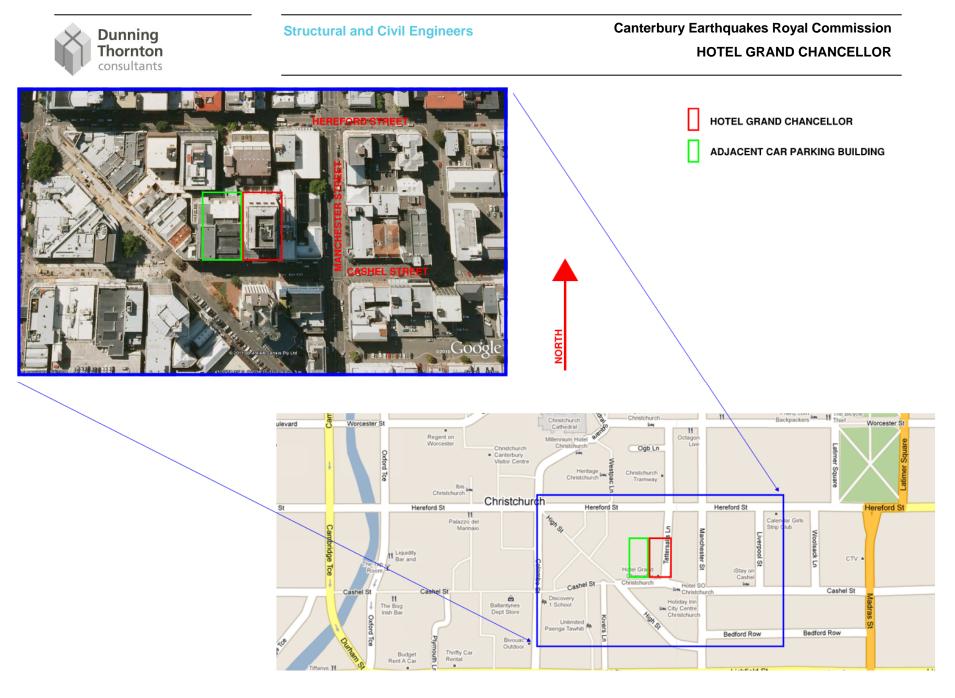
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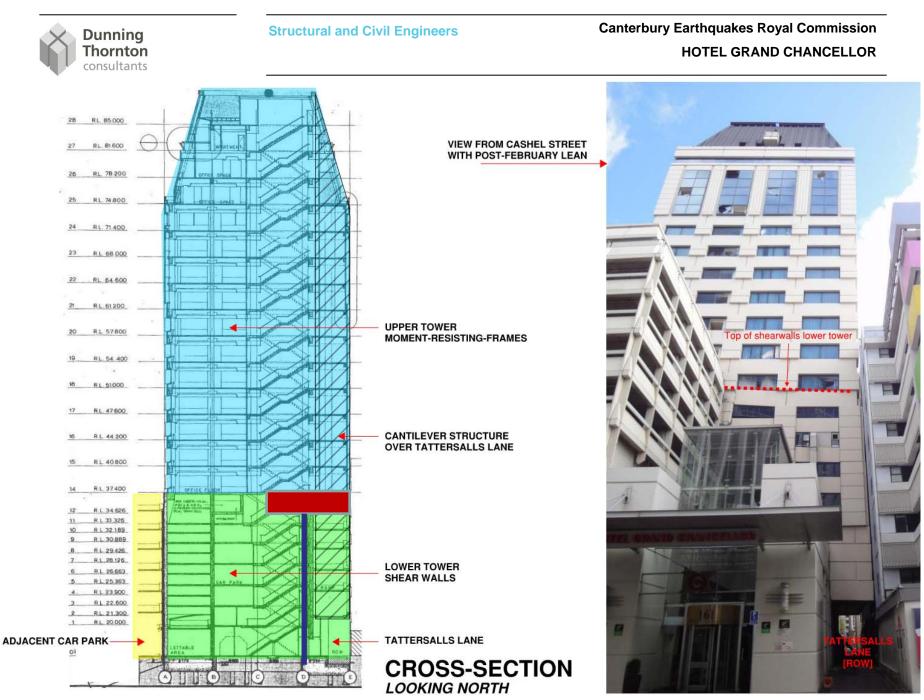
Presentation to Royal Commission 17 Jan2012



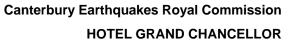
Description Of Building & Structure Nature of Failure **Structural Actions Wall D5-6 Seismicity Stairs Other Damage Questions Answered Recommendations Issues Arising from Review**

Hotel Grand Chancellor: Pre-September Earthquake (source: C Lund & Son Ltd website) Fig.1





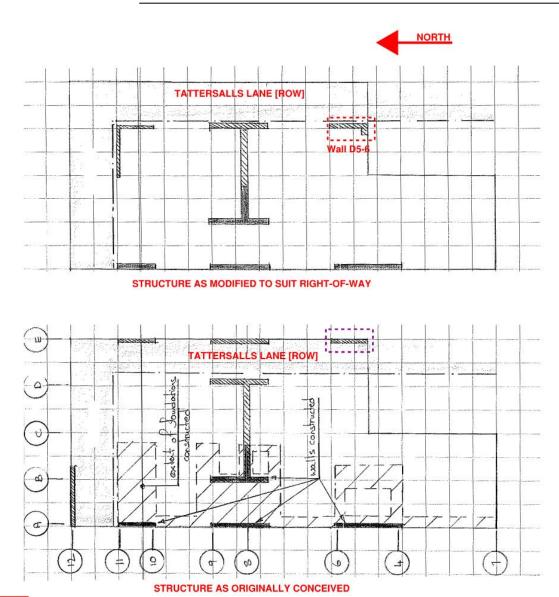






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STRUCTURAL CONCEPT

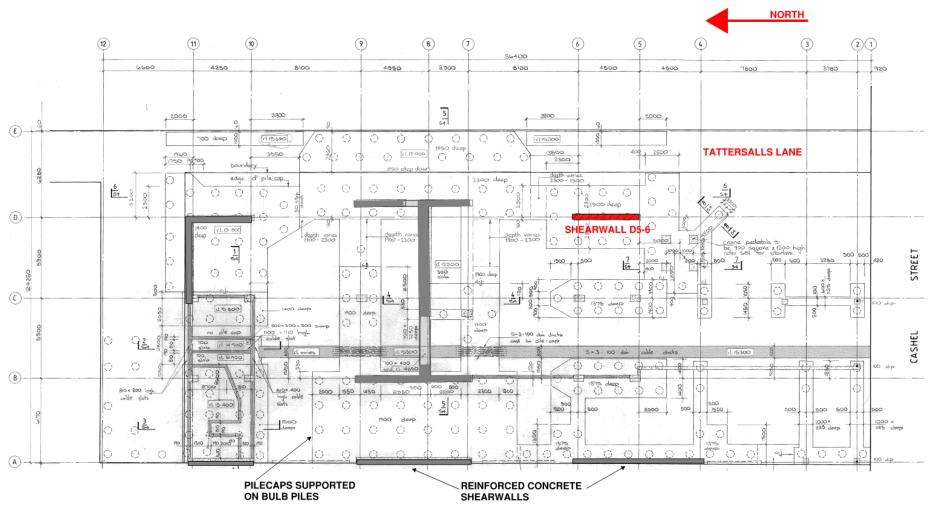
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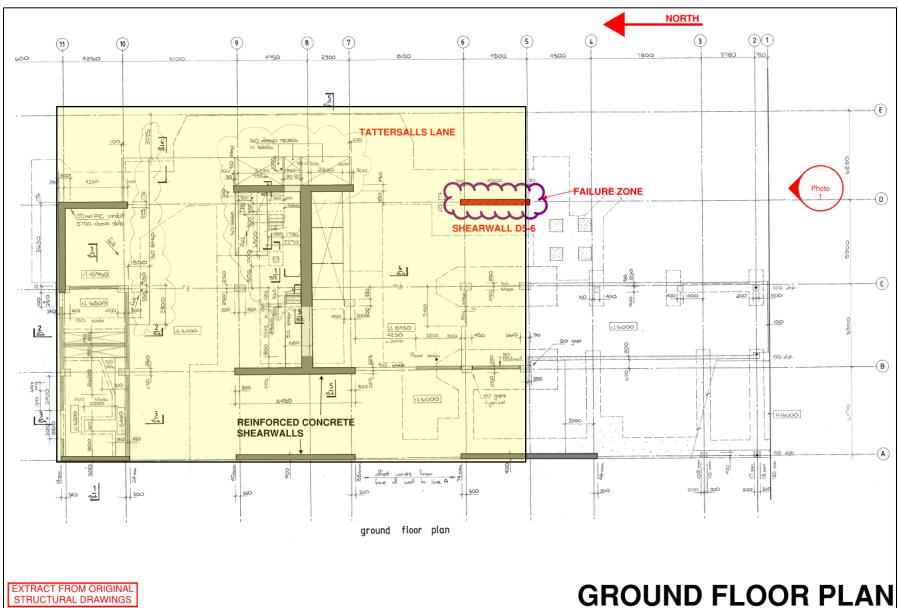


FOUNDATION PLAN

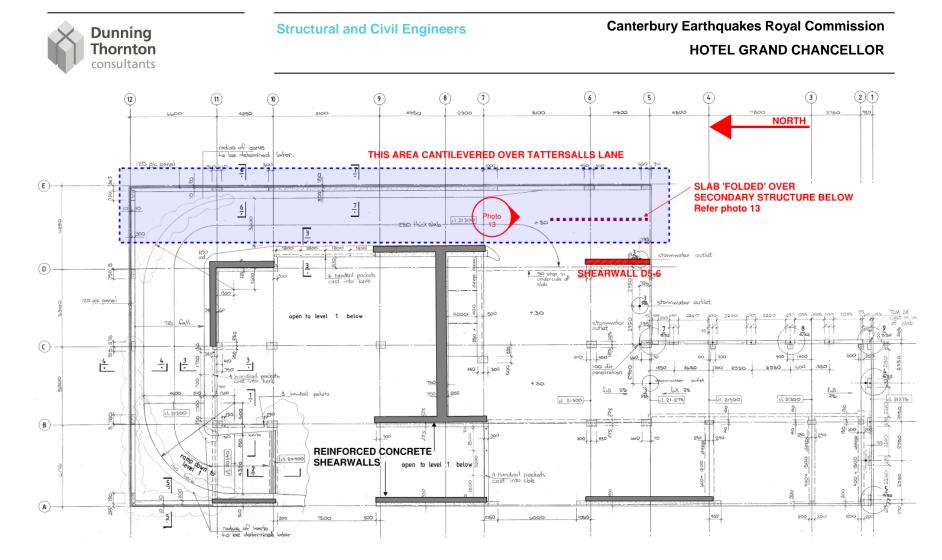


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STRUCTURAL DRAWINGS



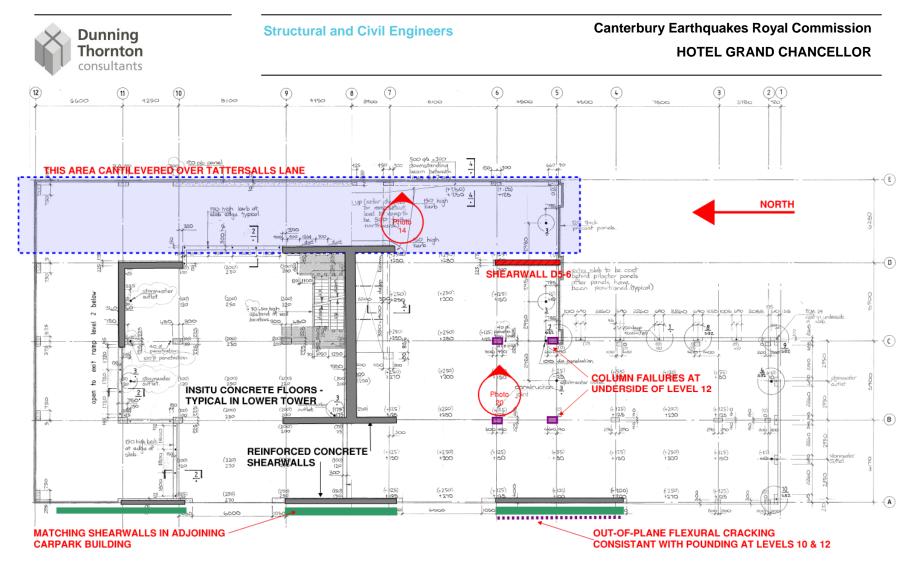
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Photo 13 - Folded slab at level 2



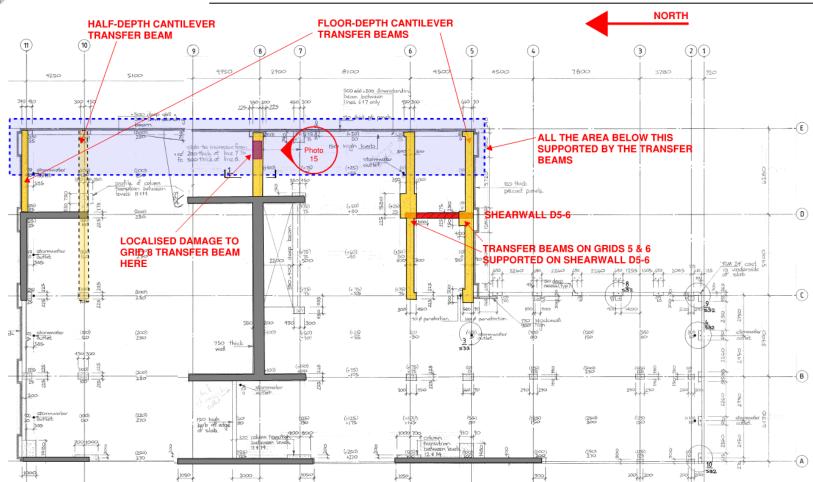
LEVELS 3 TO 8 SIMILAR

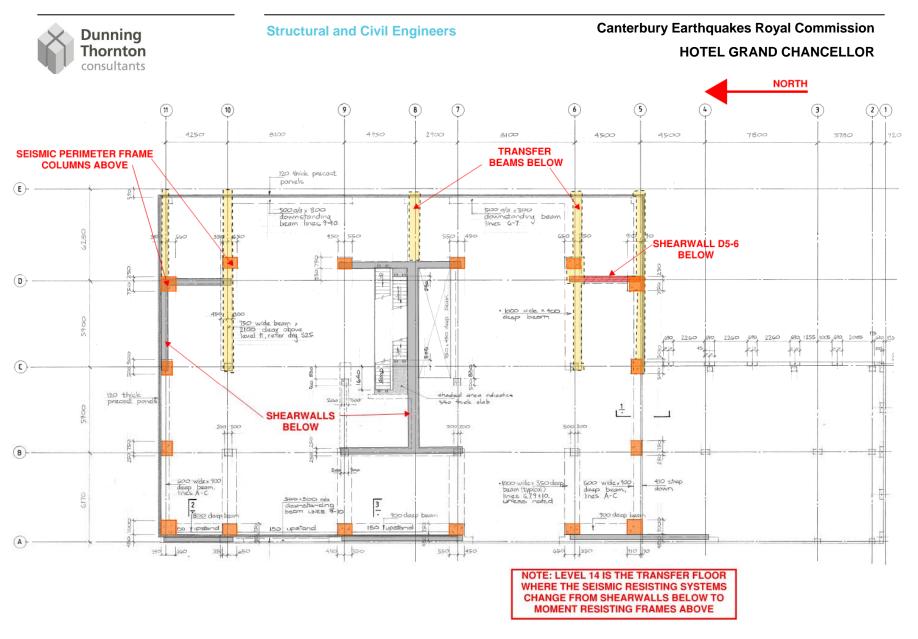
EXTRACT FROM ORIGINAL STRUCTURAL DRAWINGS Dunning Thornton consultants

Structural and Civil Engineers

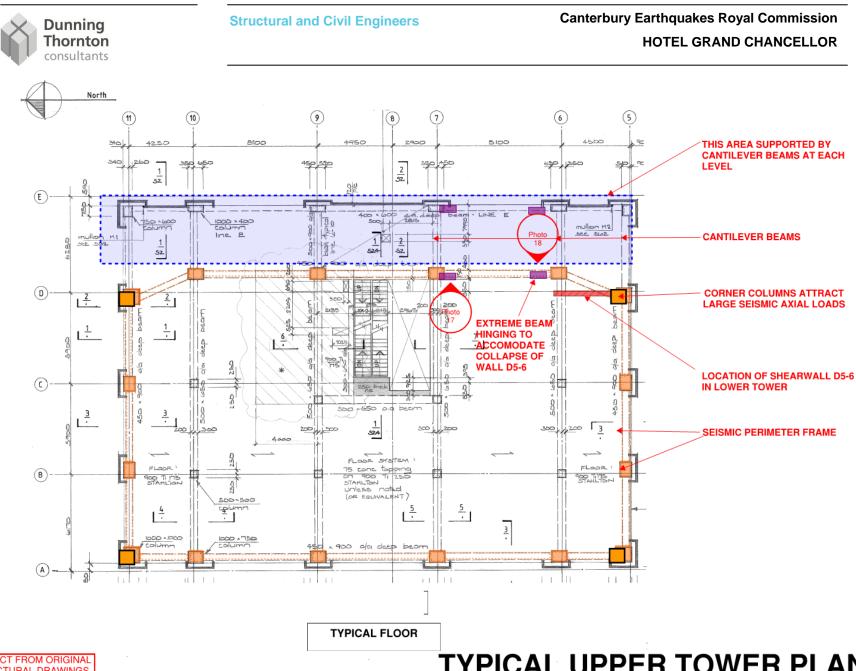
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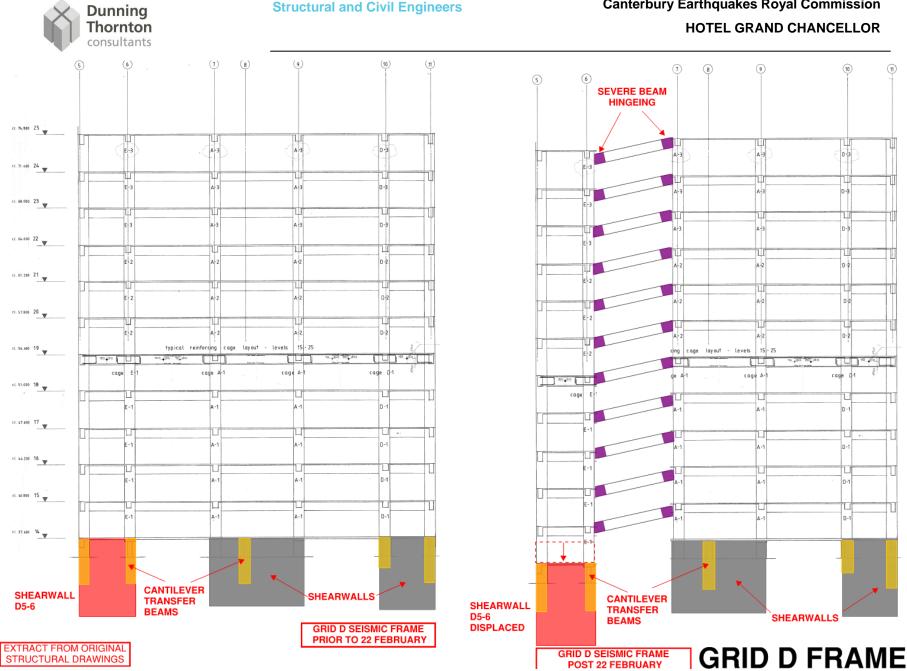
LEVEL 14



EXTRACT FROM ORIGINAL STRUCTURAL DRAWINGS

TYPICAL UPPER TOWER PLAN

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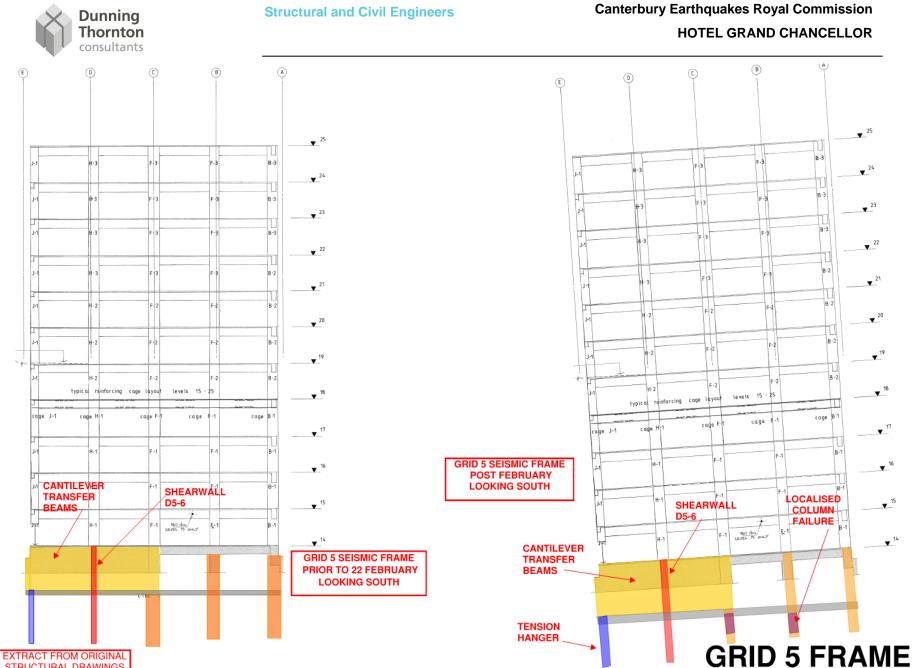




Photo 18 - Hingeing in grid D tower beams



Photo 19 - Hingeing in grid D tower beams



STRUCTURAL DRAWINGS







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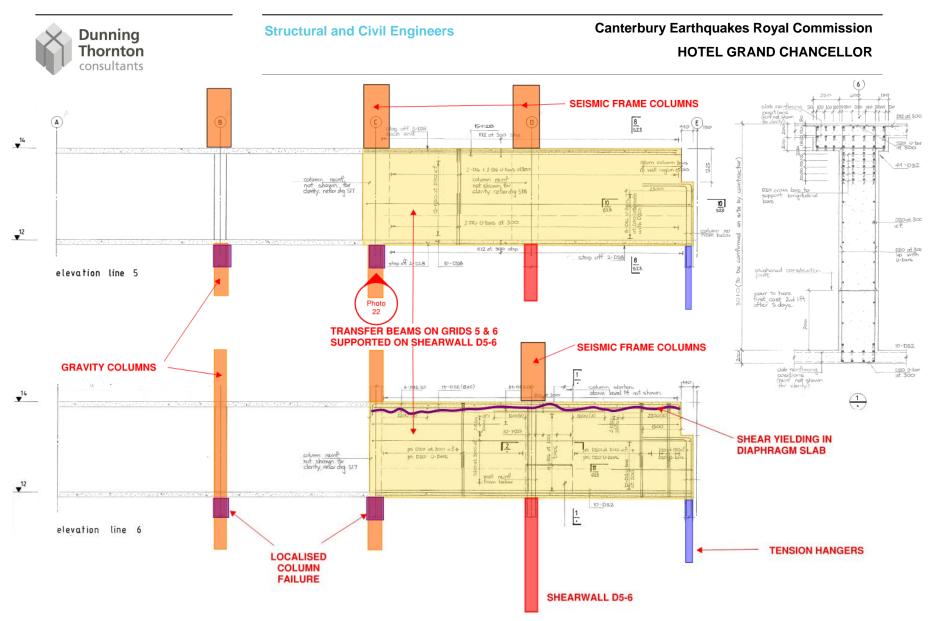


Photo 4 - Eastern Elevation - Post February



Photo 3 - Southern Elevation - Post February

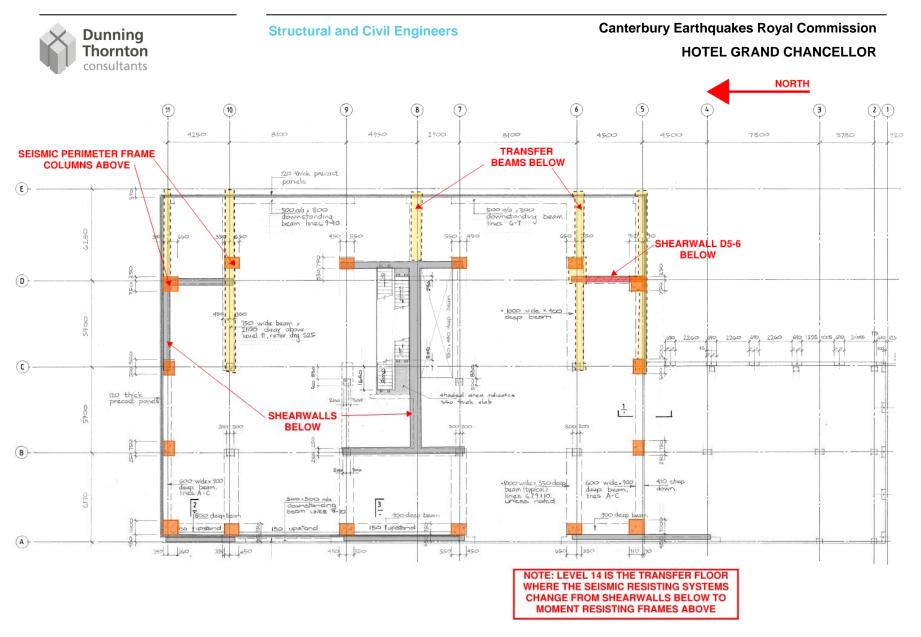
Photo 5 - Damage at junction between podium and tower not related to pounding



EXTRACT FROM ORIGINAL

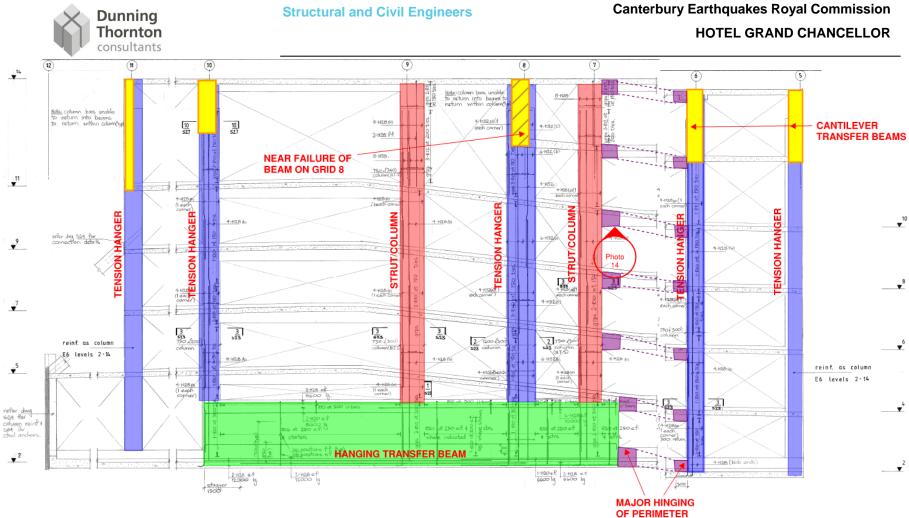
STRUCTURAL DRAWINGS

TRANSFER BEAMS GRIDS 5 & 6



LEVEL 14





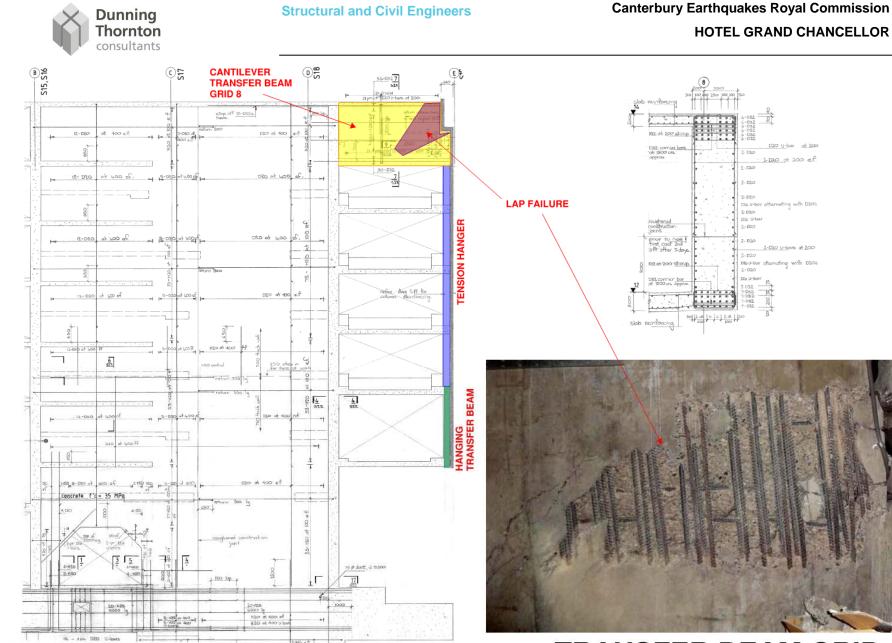
BEAM





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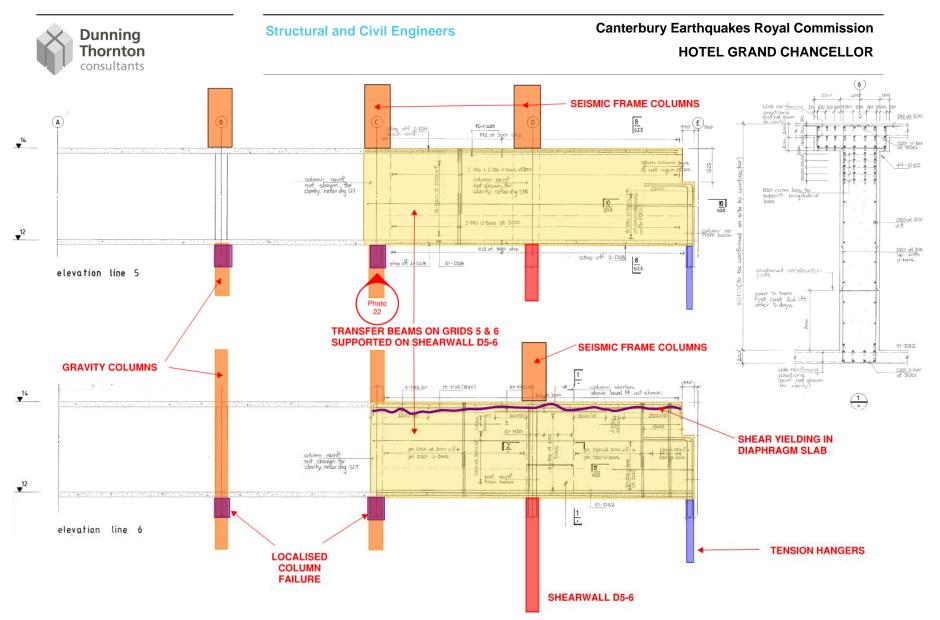




TRANSFER BEAM GRID 8

EXTRACT FROM ORIGINAL STRUCTURAL DRAWINGS

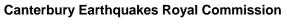
ELEVATION GRID 8



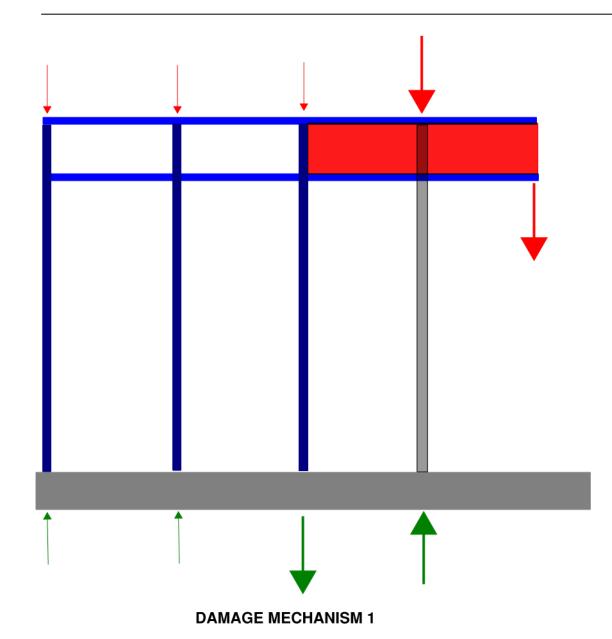
EXTRACT FROM ORIGINAL

STRUCTURAL DRAWINGS

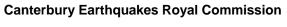
TRANSFER BEAMS GRIDS 5 & 6



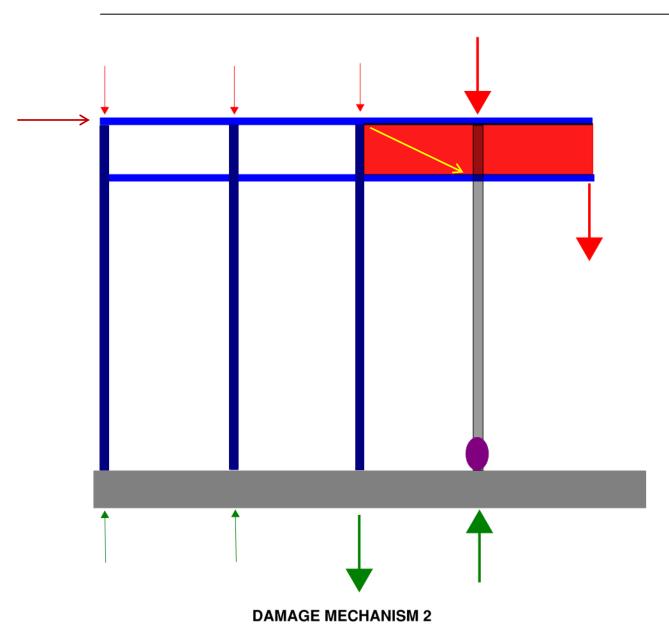
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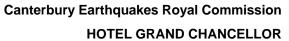


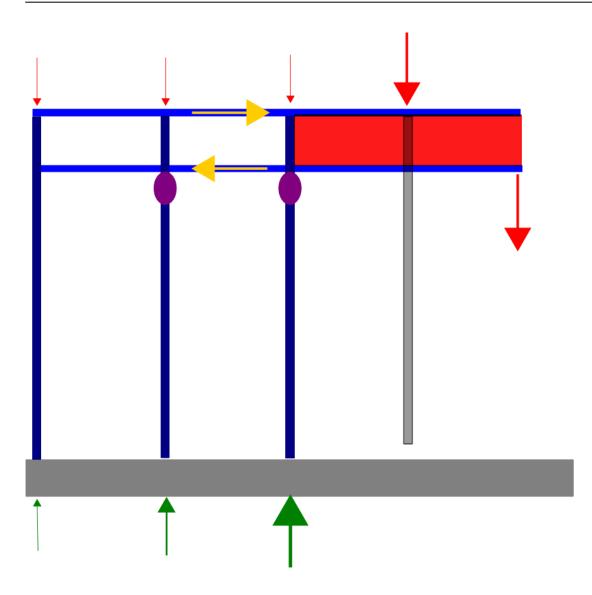


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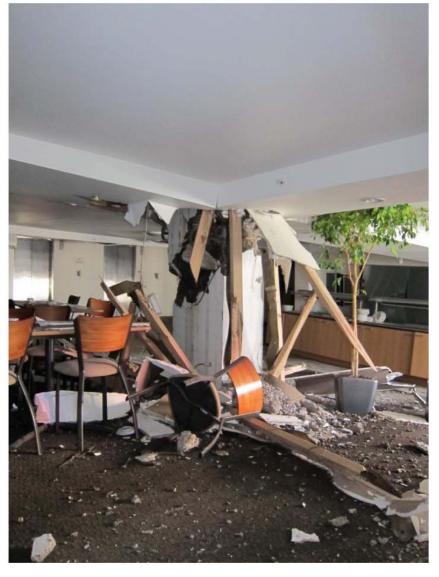


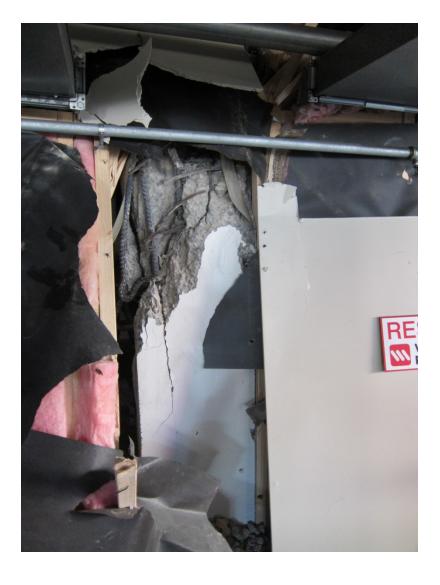
Photo 20 - Crushed columns at level 10, lines 5 & 6

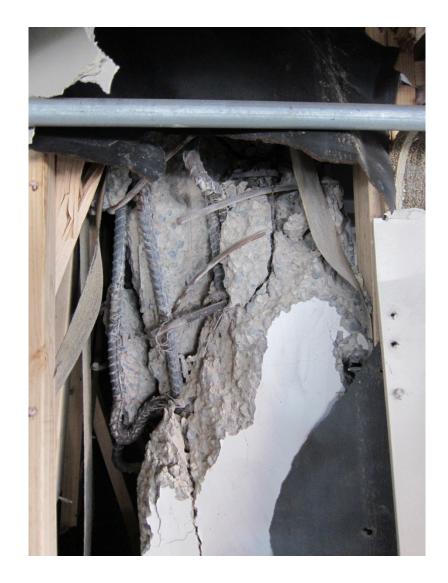
Photo 21 -Crushed columns at level 10, lines 5 & 6



Photo 22 -Crushed columns at level 10, lines 5 & 6

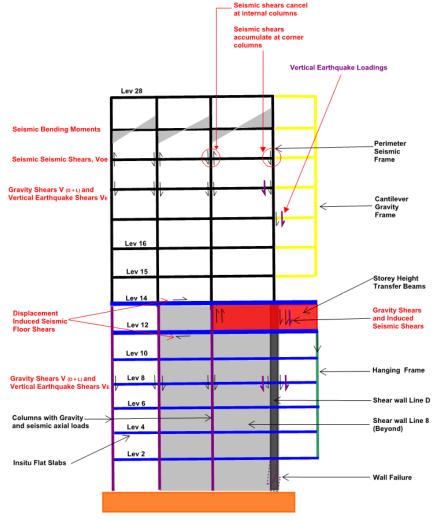








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F.1.1 Axial Actions

Derived Axial Loads (at each end of wall)	@-5D @-6D		
Gravity Loads (D+L _R)	6300kN	8500kN	
Mass contributing to Vertical Earthquake forces	450T	680T	
Range of Vertical Earthquake Loads (V _E) (range of 0.5g to 1.5g, Note NZS4203:1984 required 0.9g on parts)	2300kN to 6800kN	3400kN to 10200kN	
Seismic Overstrength Beam Shears (Voe) from upper tower	10100kN -2800kN		
Displacement Induced Seismic from transfer beams	3000kN	3000kN 3000kN	
D+1.3L + E	21700kN to 26200kN	12100kN to 18900kN	
1.4D+1.7L	8800kN	12100kN	
Total Load on Wall5-6 D+1.3L + E : (with V _{E)} : (without V _{E)} 1.4D + 1.7 L	33,800 – 45,000kN 28000kN 20,900kN		

HGC Frame Line 6 Axial Load Actions



F.1.2	2 Moment In-plane code moment <i>maximum from Etabs Analysis</i> Building over-strength of approximately 2 Assessed moment Range Out-of-plane moment				6000kNm 10-15MNm 600kNm	
F.1.3	Shear In-plane she	ear				800kN
F.1.4	Axial load	Actions – pos lane moment sign Actions Axial Load Moment Shear		<i>maximum</i> 17MN 8MNm 800kN	s	33-45MN 10-15MNm 1.5-2MN

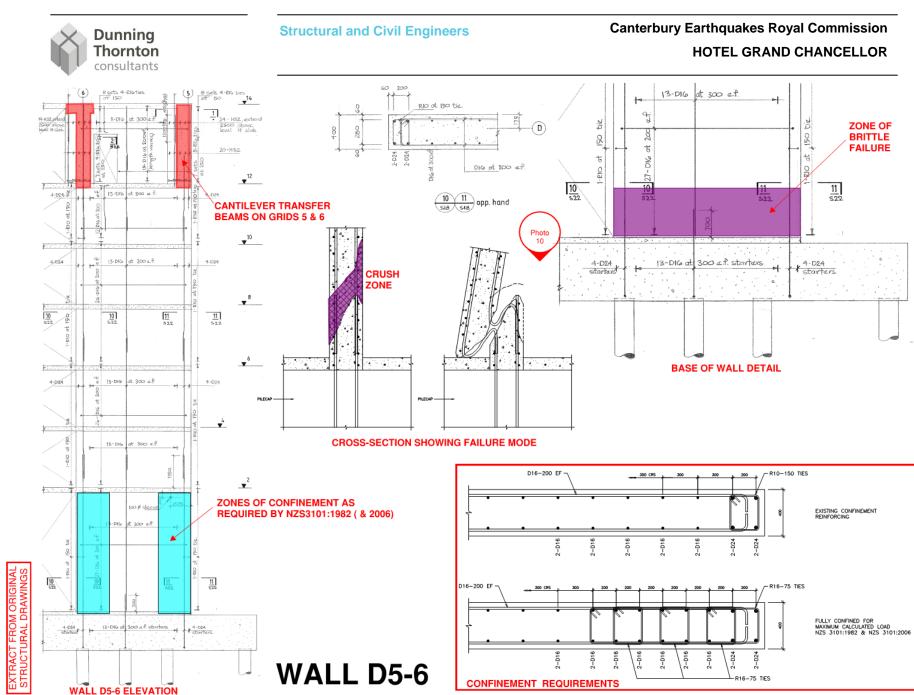






Photo 6 - Shearwall D5-6 - Base Failure

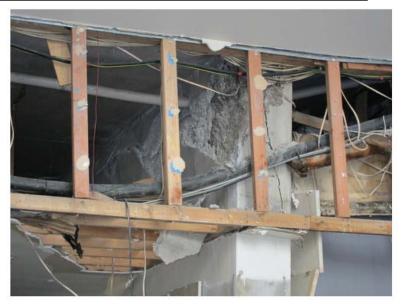


Photo 7 - Shearwall D5-6 - Hingeing at top of ground floor



Photo 8 - Shearwall D5-6 - Hingeing at top of ground floor









Photo 11 - Similar Shearwall Failure Not the Hotel Grand Chancellor

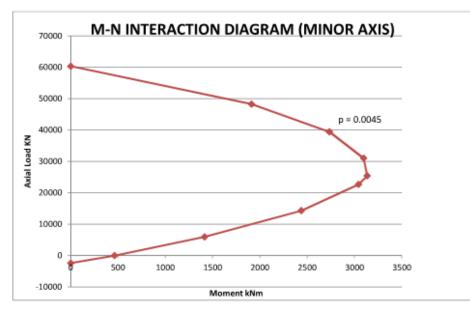


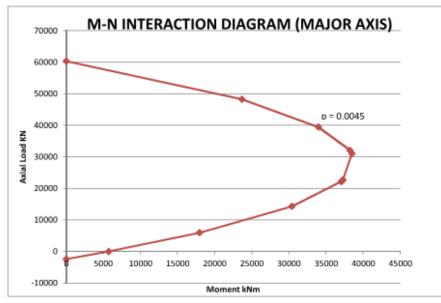
Photo 12 - Similar Shearwall Failure Not the Hotel Grand Chancellor

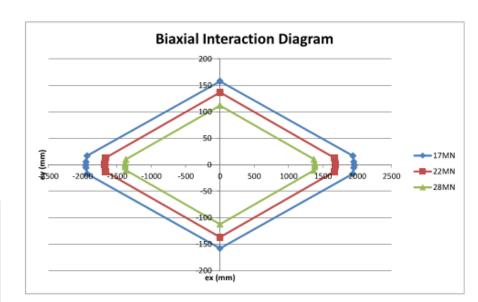


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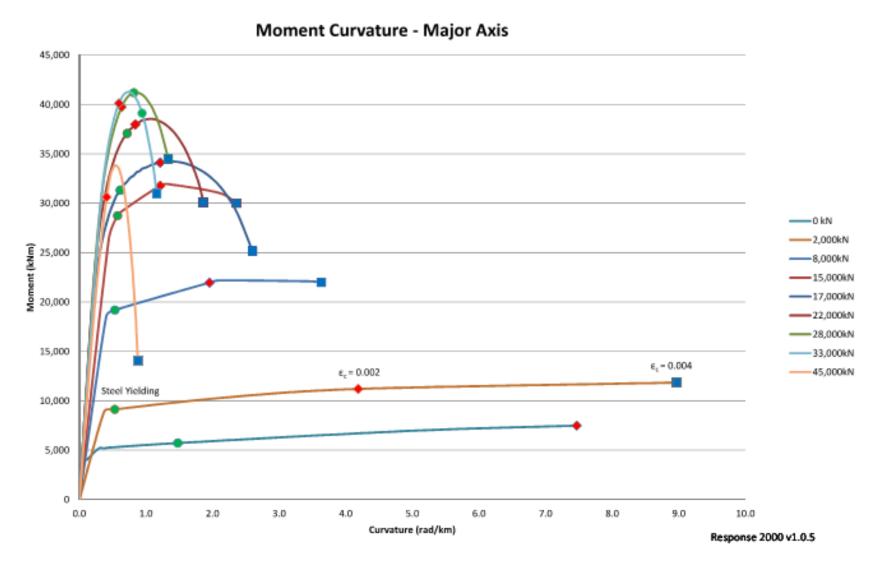






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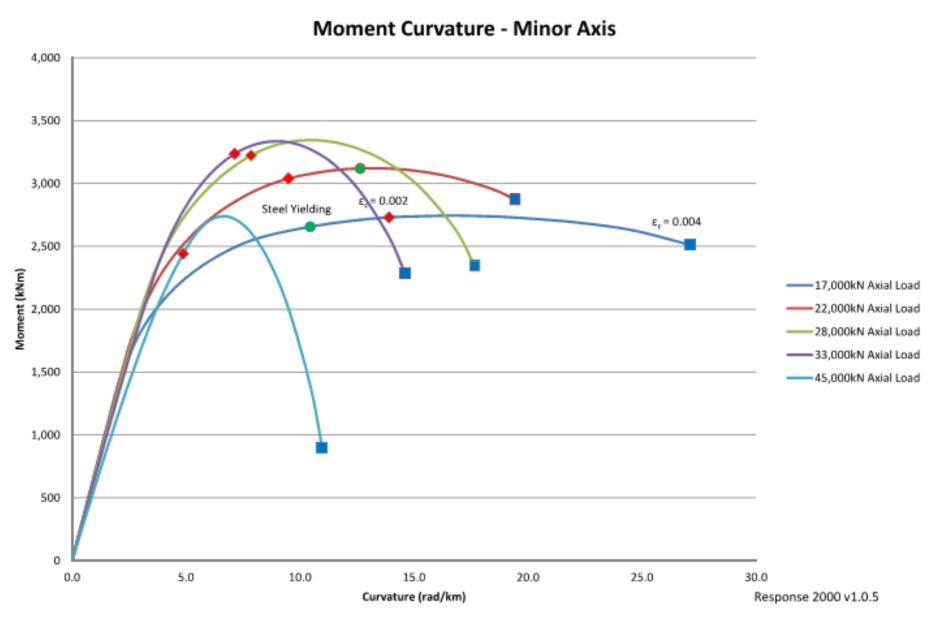


Moment Curvature - Major Axis

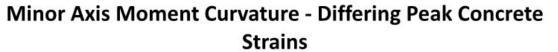


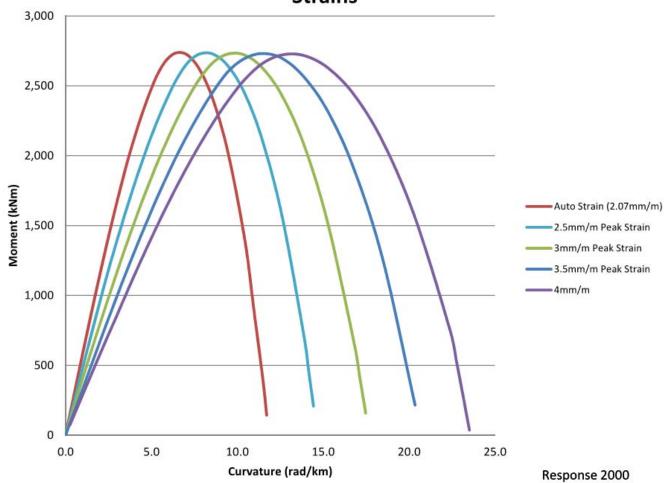
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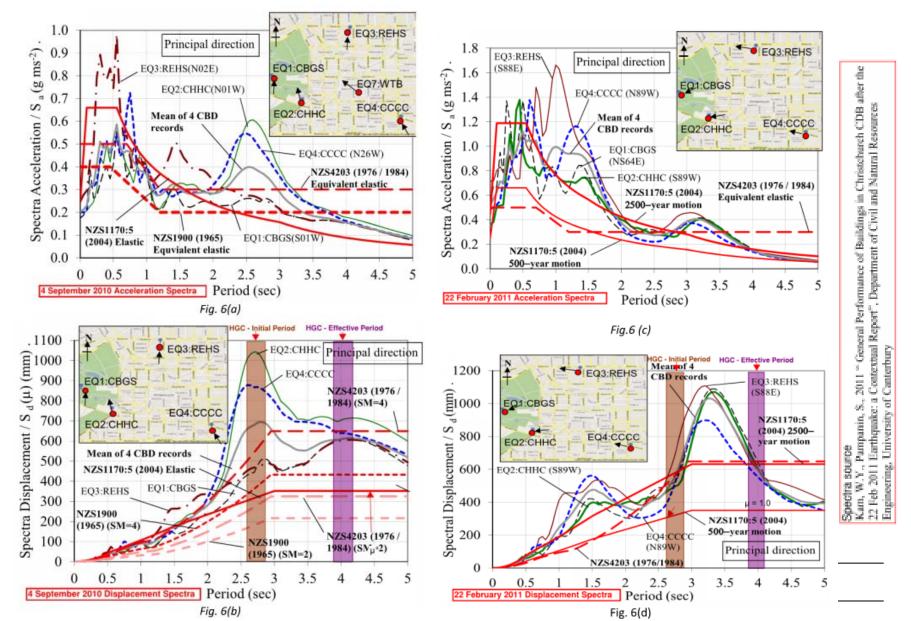




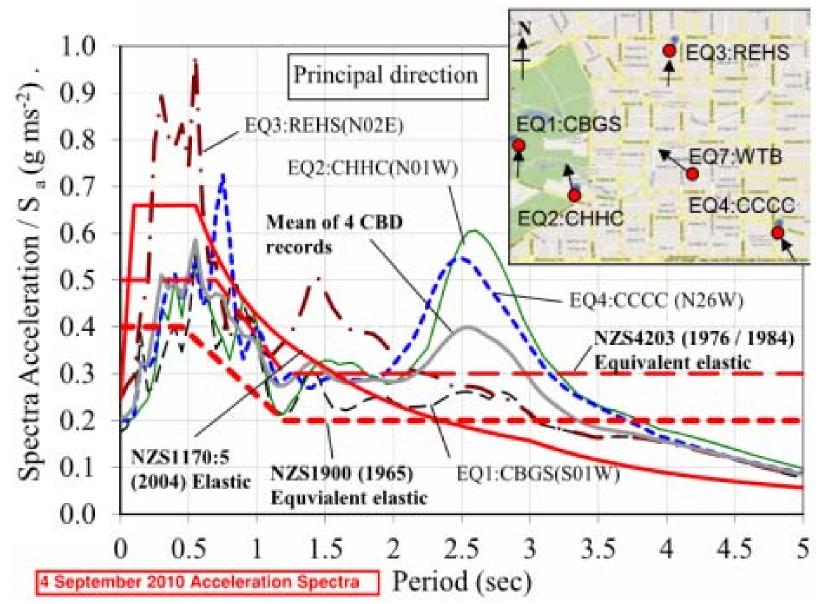


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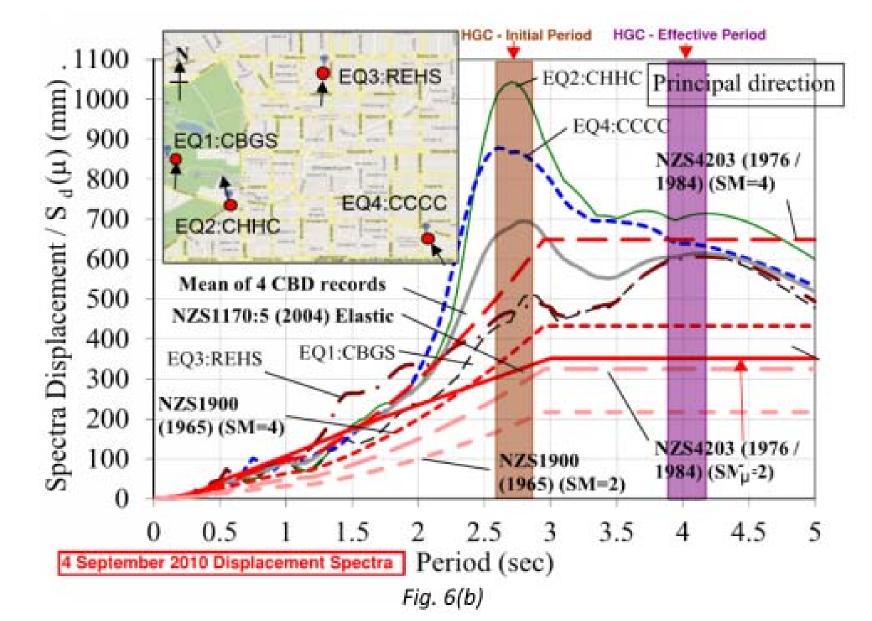
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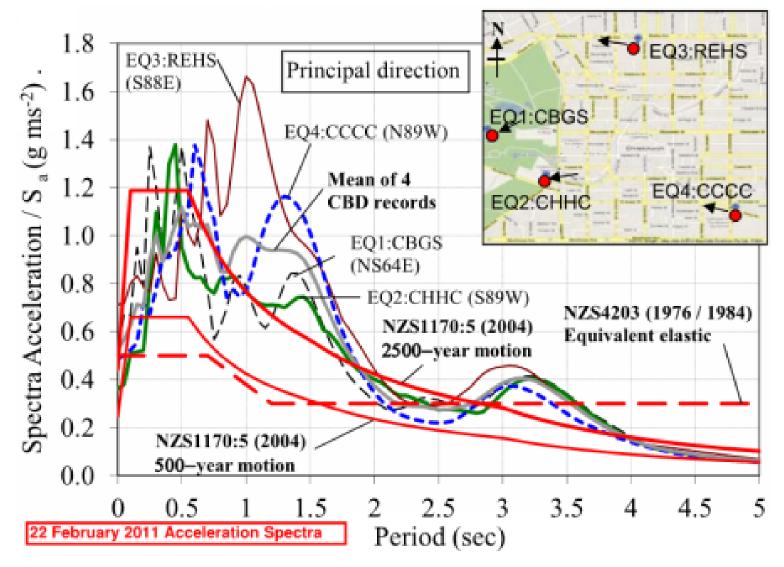
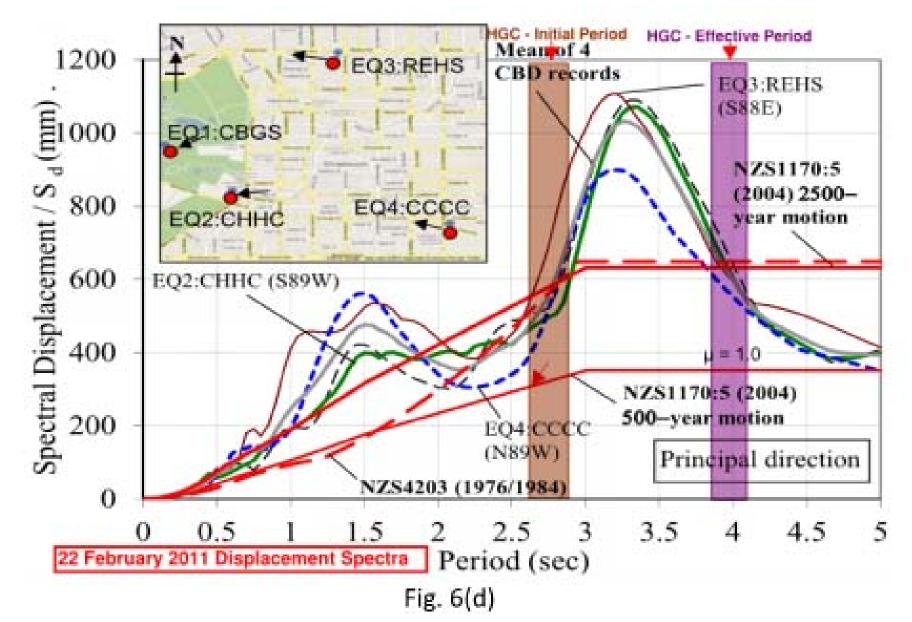


Fig.6 (c)



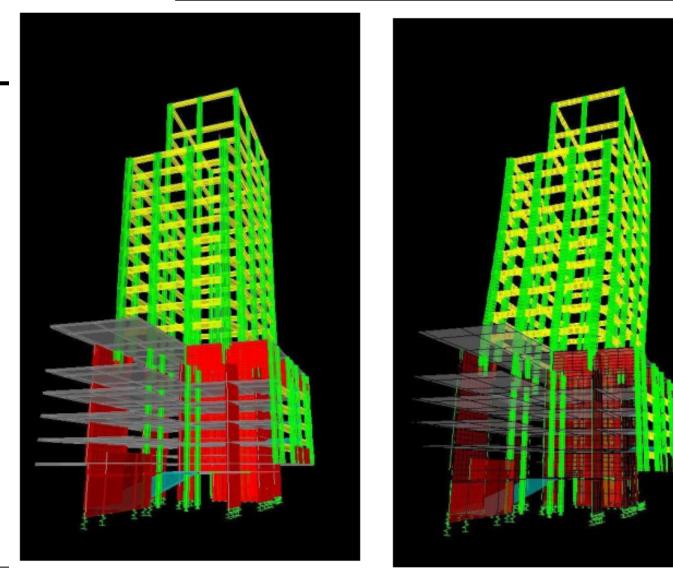
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South (Front) View

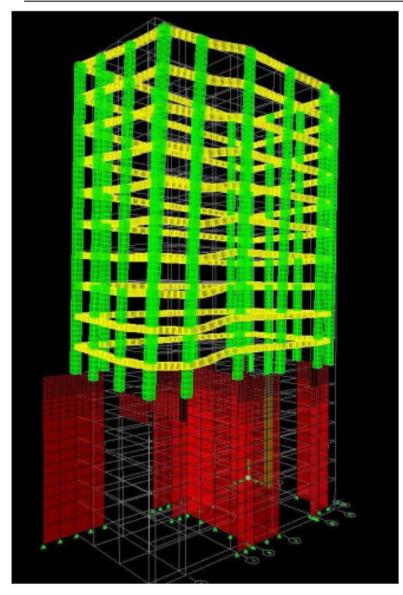
South View -Deformed



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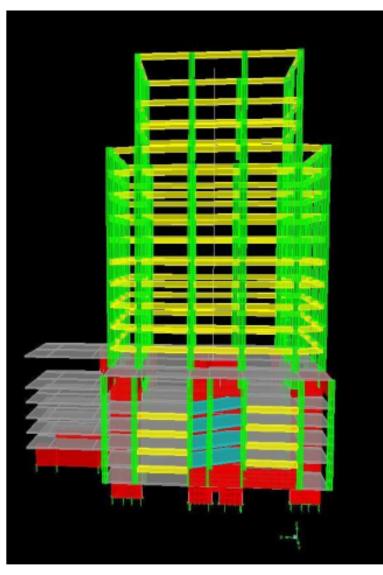
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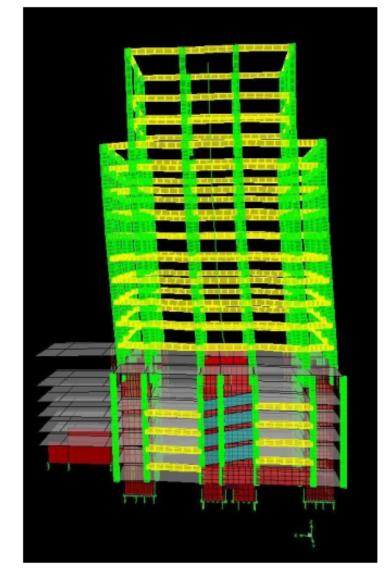


Corner Displacement



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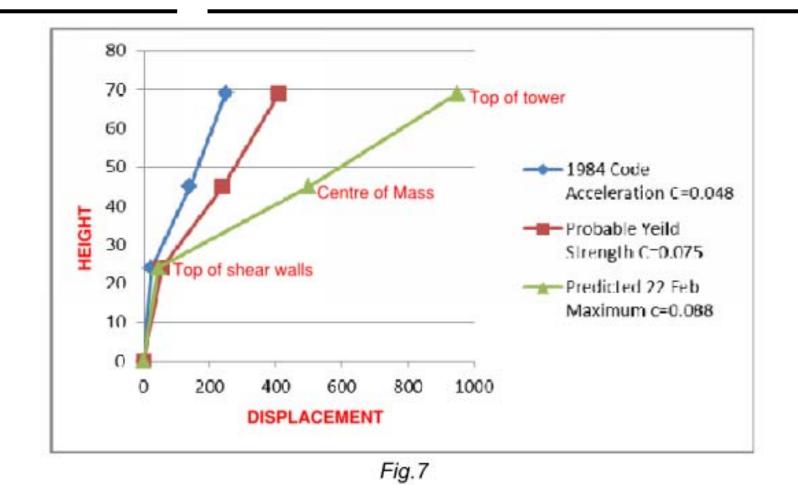


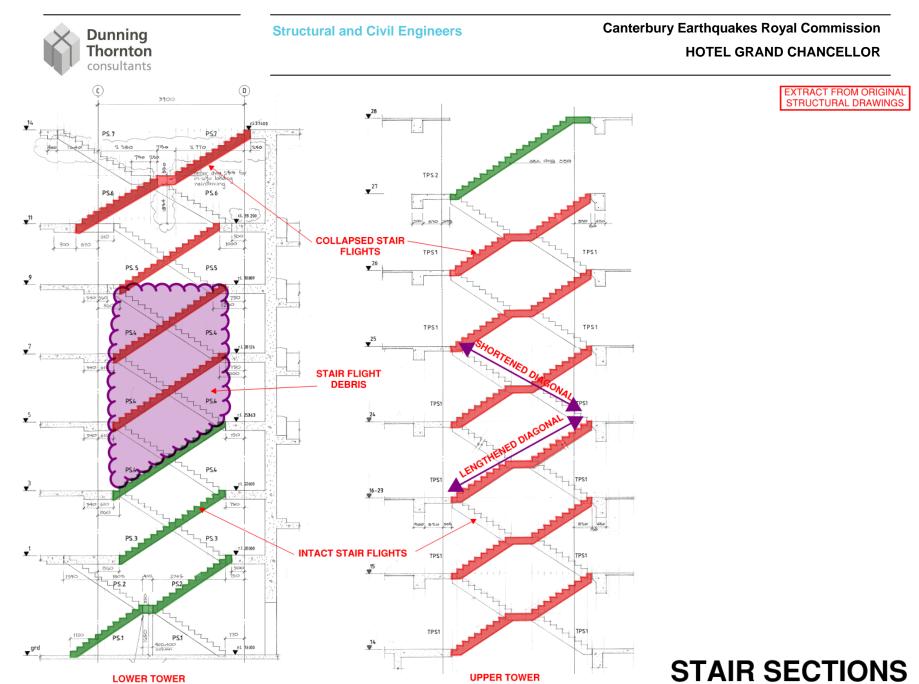


East Side View - Deformed

East Side View

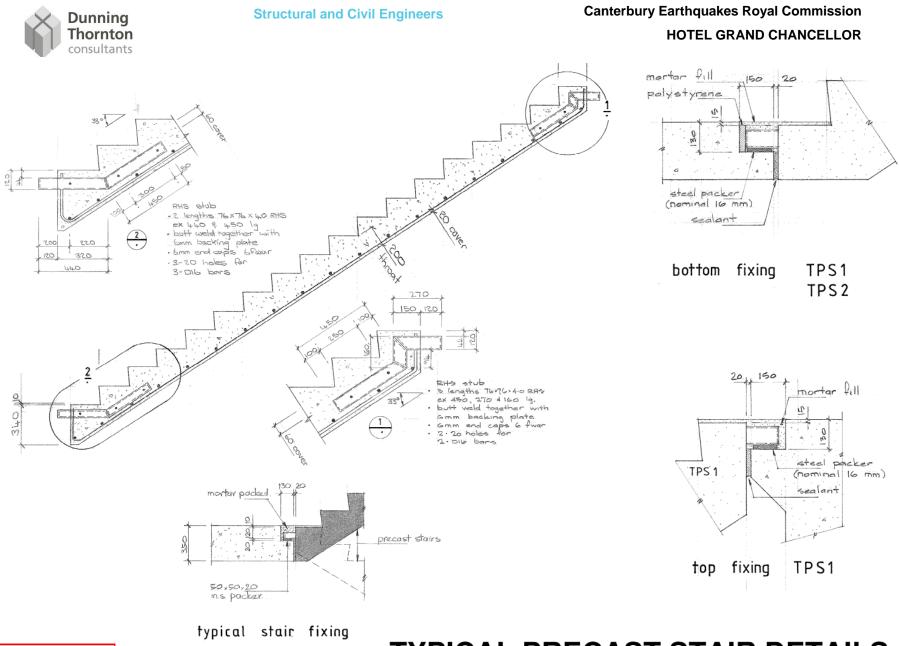






UPPER TOWER

LOWER TOWER



EXTRACT FROM ORIGINAL STRUCTURAL DRAWINGS

at floor levels

TYPICAL PRECAST STAIR DETAILS



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Photo 23 - Top of Intact stair flight

Landing damage



Photo 24 -Highest surviving stair, supporting debris

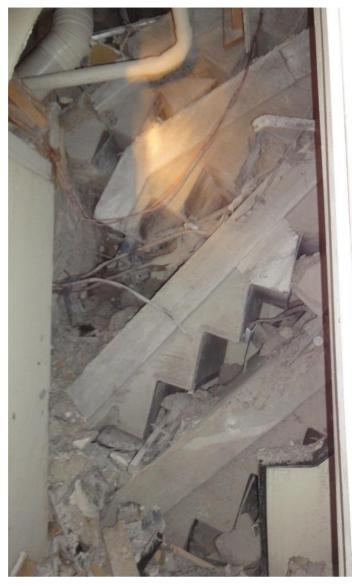


Photo 25 - Stair Debris









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Questions Answered

- Response to September Event
- Compliance with Contemporary Codes?
- Failure in a 'Code' Event?
- Failure in NZS1170.5 Event?
- %NBS?
- Stair collapse dependent on wall failure?



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Recommendations

- Design Rigour for Irregularity
- Resign Rigour for Flexural Shear Walls
- Stair Separation
- Floor Depth Walls
- Design Rigour for Displacement Induced Actions
- Frames Supported on Cantilevers
- Ratcheting



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Issues Raised In Review

- Bi-Directional Loading
- Performance of Other Walls
- Likelihood of Stair Failure
- P-Delta Effects
- Vertical Acceleration Effects
- Speed of Loading
- Ratcheting